

72-996

KLEINMAN, Stephen A., 1941-
A REAL-TIME ADVISORY SIGN CONTROL SYSTEM
FOR URBAN HIGHWAYS.

The City University of New York, Ph.D., 1971
Engineering, automotive

University Microfilms, A XEROX Company, Ann Arbor, Michigan

A REAL-TIME ADVISORY SIGN CONTROL SYSTEM
FOR URBAN HIGHWAYS

by
Stephen Kleinman

A dissertation submitted to the Graduate
Faculty in Engineering in partial fulfill-
ment of the requirements for the degree of
Doctor of Philosophy, The City University
of New York.

1971

This manuscript has been read and accepted for the Graduate Faculty in Engineering in satisfaction of the dissertation requirement for the degree of Doctor of Philosophy.

May 19, 1971
date

Richard Wiener
Chairman of Examining Committee

19 May 1971
date

[Signature]
Executive Officer

Prof. Morris Ettenberg

Prof. Stanley Katz

Prof. Ralph Mekel

Prof. Richard Wiener, Chairman
Supervisory Committee

The City University of New York

PLEASE NOTE:

Some Pages have indistinct
print. Filmed as received.

UNIVERSITY MICROFILMS

ACKNOWLEDGMENTS

I wish to express my gratitude to those who have contributed in their various ways to the completion of this research.

I thank my parents who encouraged me to continue my education and assisted me financially. I am grateful to my wife, Paula, for her encouragement and understanding during the years of my doctoral work.

I sincerely thank my adviser, Professor Richard Wiener, for his guidance and support during the progress of this research. His interest in my work and his many helpful technical suggestions are greatly appreciated.

To Mr. Mitch Haspel, for his technical advice, I express my thanks.

To Miss Sadie Silverstein, for her outstanding job of typing this dissertation, I express my appreciation.

This research was partially supported by The New York State Science and Technology Foundation under Grant SSF(8)15.

TABLE OF CONTENTS

Chapter		Page
1	INTRODUCTION	1
2	SIMULATION PROGRAM	9
2.1	Justification for Simulation	9
2.2	The Simulation Program	11
	Simulation Approach	11
	The Simulated System	12
	Input Parameters	13
	Simulated Output	14
	Storage of Variables	17
	Generation of Vehicles	21
	Motion of First Vehicle	22
	Dynamics of Following Vehicles	23
	Advisory Sign Settings	23
3	MODELS OF VEHICLE DYNAMICS	24
3.1	Models of Vehicle Dynamics	24
	Introduction	24
	Models of Single-lane Traffic -- Historical Background	24
	Limitations of Basic Car-following Models	25
	Formulation of the Simulation Models	26
	The Car-following Mode	27
	Car-following Deceleration Model	28
	Car-following Acceleration Model	31
	The Distance-detection Mode	32
	The Headway Factor	34
	Sign-following	35
	Overall Acceleration	36
3.2	Generation of Headway Factors	37

4	DERIVATION AND APPLICATION OF THE SIGN CONTROL ALGORITHM	52
4.1	Acceleration Noise as an Index of Performance	52
4.2	Calculation of Optimum Deceleration for a Simple Highway Situation	56
4.3	The Sign Control Algorithm as Applied to a Simple Highway Situation	68
	Assumptions	68
	Computation of Desired Deceleration	69
	Predicted Average Deceleration from Sign-following Model	73
	Solution for Sign Setting	78
	Rounding of Sign Settings	78
	Example of Sign Settings for a Simple Highway Situation	79
4.4	Application of Sign Control to a General Highway Situation	82
	Introduction	82
	General Procedure for Sign Setting	82
5	SIMULATION RESULTS AND DISCUSSION	86
5.1	Prefatory Remarks	86
5.2	Maneuvers of the Lead Vehicle	88
5.3	Numerical Results	90
5.4	Recommendations for Further Study	95
	APPENDIX A FLOW CHARTS OF THE COMPUTER SIMULATION PROGRAM	112
	APPENDIX B LISTING OF FORTRAN SIMULATION PROGRAM	123
	REFERENCES	174
	AUTOBIOGRAPHICAL STATEMENT	178

LIST OF TABLES

Number		Page
4-1	Value of the integral I for optimal and sub-optimal waveforms	64
5-1	Results without sign control and with sign control at 100% compliance, for D = 2300 veh/hr	97
5-2	Results without sign control and with sign control at 100% compliance, for D = 2200 veh/hr	98
5-3	Results without sign control and with sign control at 100% compliance, for D = 2100 veh/hr	99
5-4	Results without sign control and with sign control at 100% compliance, for D = 2000 veh/hr	100
5-5	Results without sign control and with sign control at 100% compliance, for D = 1900 veh/hr	101
5-6	Results without sign control and with sign control at 100% compliance, for D = 1800 veh/hr	102
5-7	Results without sign control and with sign control at 100% compliance, for D = 1700 veh/hr	103
5-8	Results without sign control and with sign control at 100% compliance, for D = 1600 veh/hr	104
5-9	Results without sign control and with sign control at 100% compliance, averaged over several driver samples, for maneuver 1	105
5-10	Results without sign control and with sign control at 100% compliance, averaged over several driver samples, for maneuver 2	106
5-11	Average duty cycle of advisory signs at 100% compliance	107

5-12	Results at various compliance levels, for maneuver 1, for D = 2300	108
5-13	Results at various compliance levels, for maneuver 1, for D = 2200	108
5-14	Results at various compliance levels, for maneuver 1, for D = 2100	109
5-15	Results at various compliance levels, for maneuver 1, for D = 2000	109
5-16	Results at various compliance levels, for maneuver 1, for D = 1900	110
5-17	Percent change in acceleration noise due to sign control at various compliance levels, for maneuver 1	110

LIST OF FIGURES

Number		Page
2-1	Storage Layout	19
3-1	Anticipatory factor used in deceleration model	30
3-2	Headway factor density function	40
3-3	Headway factor distribution function	42
3-4	Headway factor density function for $D = 1800 \text{ veh/hr}$, $V = 50 \text{ mi/hr}$	42
3-5	Probability that headway factor is less than 1 vs. HMAX	44
3-6	Probability that headway factor is less than 1 vs. D , for $V = 50 \text{ mi/hr}$	44
3-7	Maximum headway factor vs. D and maximum time headway vs. D , for $V = 50 \text{ mi/hr}$	45
3-8	Probability that headway is less than mean headway vs. D , for $V = 50 \text{ mi/hr}$	48
3-9	Headway density function vs. D	49
3-10	Generating samples from $f(h)$	51
4-1	Acceleration waveforms	54
4-2	The integral $I(T)$	59
4-3	Speed profile for $T/3 < t_0 < T$	59
4-4	Acceleration waveform for $T > 3D/V_0$	59
4-5	Optimal motion	61
4-6	Acceleration waveforms for optimal and sub- optimal solutions	63

4-7	Speed of following vehicle for constant deceleration case	66
4-8	Representation of highway and location of detectors	70
4-9	Detector speed outputs for a typical case	83
5-1	Lead vehicle motion	89
5-2	Percent decrease in acceleration noise due to sign control at various compliance levels, for maneuver 1	111

Abstract

A REAL-TIME ADVISORY SIGN CONTROL SYSTEM FOR URBAN HIGHWAYS

A real-time computer controlled advisory speed sign system is proposed in order to improve the level of comfort and safety on an urban expressway at high traffic volumes. By transmitting advance warnings of impending slowdowns the system attempts to prevent sudden sharp decelerations caused by the amplifying effect of car-following. The control system output consists of command settings for the advisory signs located along the highway at intervals of one-tenth mile. Roadbed detectors continually monitor and transmit speed and density data to the on-line computer. The processing of this information in accordance with a control algorithm results in command settings for the advisory speed signs.

For practical reasons the sign settings are restricted to the usual 5 mi/hr increments and a particular sign is set only if a significant response is necessitated thus insuring the highest possible compliance. The effect of partial compliance is investigated by varying the percentage of drivers that heed the advisory sign settings.

The sign control algorithm is fashioned to minimize the total average acceleration noise for all vehicles. Based on a calculus of variations solution to a simplified highway situation a desired average

deceleration is determined for vehicles on each one-tenth mile section of highway upstream of the disturbance. The desired deceleration is expressed in terms of the measured section speeds and densities. Sign settings are chosen so that the predicted average vehicle deceleration on each section equals the desired deceleration. The predicted average deceleration is derived from an assumed sign-following law as a function of a car's speed when entering the section and the speed setting of the advisory sign located at the next section boundary.

An elaborate computer simulation which incorporates the detectors, signs, sign control algorithm, and the vehicles as they respond to other cars and advisory signs is developed. Flow disturbances are programmed in order to test the effectiveness of the sign control algorithm. The significant reduction in acceleration noise, even for partial compliance, indicates that meaningful improvements in safety and comfort can be achieved by advisory sign control.

CHAPTER 1

INTRODUCTION

In recent years the highways in and around metropolitan areas have become increasingly subject to congestion. In the vicinity of urban areas it is neither feasible nor desirable to eliminate congestion by construction of additional highways. The cost of rights-of-way and the social impact of displacing large numbers of people are prohibitive. Yet the penalty in terms of increased travel times, frazzled nerves, and higher accident rates that are characteristic of congested traffic mandates a solution.

To this end considerable sums of money have been spent to sponsor theoretical and experimental studies of the various aspects of traffic flow. This research has resulted in a better understanding of the dynamics of traffic flow and in many cases has led to implementation of surveillance and control systems that have improved the quality of flow.

As of the beginning of 1970, seventeen of the States and Washington, D.C., have implemented some form of freeway surveillance and control system¹. Nine of the ten most populous states are in this category.

The basic goals of these systems are (1) to prevent congestion, (2) to communicate alternative route and speed information to motorists, (3) to provide rapid detection of accidents, and (4) to provide service to stranded motorists.

Aside from accidents and disabled vehicles, the most severe disruptions to expressway flow occur because the demand is greater than the capacity. Thus, quite appropriately, attempts to prevent congestion in tunnels and on expressways have concentrated on limiting the rate at which vehicles enter a section of roadway so as to keep the flow rate downstream from an entrance less than the section's capacity.

Initial efforts to control traffic utilizing both a theoretical and experimental approach were made by Edie and Foote^{2,3,4} of the Port of New York Authority. They attempted to increase the flow in the Lincoln Tunnel by restricting the vehicle input rate, on a two minute basis, to be less than the capacity of the tunnel bottleneck. The tunnel controls have increased in sophistication over the years, progressing from manual to digital computer control⁵. Their system has achieved an increase in flow of between five and ten percent and has dramatically reduced automotive exhaust emissions and stoppages inside the tunnel.

Entrance ramp control, in one of its several forms⁶, is currently the most effective method of preventing congestion on urban expressways. In its simplest form it consists of closing selected entrance ramps during peak hours. The ramp metering method utilizes a traffic signal light at the entrance ramp to control the ingress of vehicles. The input rate is either determined a priori based on historical data or computed in real-time from traffic measurements. A more sophisticated version of ramp metering incorporates gap acceptance merging^{7,8,9} which minimizes disturbances in

the vicinity of entrance ramps by releasing vehicles only when sufficiently large gaps in the traffic stream are available. Pacer merging control systems consist of a sequence of lights that display a moving green signal to assist vehicles in merging into available gaps.

The major freeway surveillance and control projects have been implemented on the following highways:

- (1) Eisenhower Expressway^{10, 11, 12} formerly the Congress Street Expressway (Chicago, Illinois)
- (2) Gulf Freeway^{6, 13} (Houston, Texas)
- (3) John C. Lodge Freeway^{14, 15} (Detroit, Michigan)

These three control projects have each been successful in improving the level of service of the highway.

Currently the only dynamic traffic-responsive method being employed to regulate the flow on heavily traveled expressways is entrance ramp metering. However entrance ramp metering is limited in that it exerts no control over vehicles already on the roadway. Under dense traffic conditions it is all too well known that the unpredictable response of an inattentive driver can initiate a shock wave that causes upstream traffic to sharply decelerate or come to a prolonged standstill. Such an occurrence, which cannot be substantially mitigated by ramp control, produces severe strain and discomfort among drivers and increases the probability of an accident.

The potential instability of dense highway traffic is due in part to the driver reaction time. A system that transmits advance warning of

impending decelerations would be a stabilizing influence and would substantially reduce rear-end accidents. The effect of advance warning on accident rate has been demonstrated by a project on Interstate Highway No. 5 in Portland, Oregon. There a traffic actuated "PREPARE TO STOP" sign is used on the approach to a drawbridge to warn motorists (who are traveling at 70 mi/hr) of when a long queue of stopped vehicles is ahead. Galvanic skin reflex studies conducted by the Bureau of Public Roads¹⁶ have shown that driver tension, and therefore discomfort, decreases as predictability of interferences increases. Michaels and Solomon tested a vehicle mounted signal system¹⁷ that provided advance information on the magnitude of an acceleration or deceleration. They found that car-following behavior was significantly affected with this system. A driver's headway exhibited less variability.

In this research a particular system capable of dampening flow disturbances by transmitting to drivers advance warning of impending slowdowns is proposed and investigated. The project has been motivated by the belief that a properly designed advance warning system will improve the driver's level of comfort and reduce the number of highway accidents.

The type of advance warning system that has been investigated in this research is a real-time computer controlled advisory speed sign system. The control system output consists of command settings for the advisory signs located along the highway at intervals of one-tenth mile. Roadbed detectors continually monitor and transmit speed and density data

to an on-line computer. The processing of this information in accordance with a control algorithm results in command settings for the advisory speed signs. For practical reasons the sign settings are restricted to 5 mi/hr increments and the rate at which signs are changed is such that a driver is not presented with a rapidly varying sequence of sign settings. In addition, a particular sign is set only if a significant response is necessitated thus ensuring the highest possible compliance. Compliance will be further improved because any driver who slows down in response to a sign forces the following drivers to decelerate.

Since it has been shown¹⁸ that a quantitative measure of driver comfort and safety is the acceleration noise,

$$\sigma^2 = \frac{1}{T} \int_0^T [a(t)]^2 dt$$

the sign control algorithm is fashioned to minimize the total average acceleration noise for all vehicles. Based on a calculus of variations solution to a simplified highway situation, a desired average deceleration is determined for vehicles on each one-tenth mile section of highway upstream of the slowdown. The desired deceleration is expressed in terms of the measured section speeds and densities. Sign settings are chosen so that the predicted average vehicle deceleration on each section equals the desired deceleration. The predicted average deceleration is derived from an assumed sign-following law as a function of a car's speed when entering the section and also the speed setting of the advisory sign located at the next section boundary.

An elaborate computer simulation which incorporates the detectors, signs, the sign control algorithm, and the vehicles as they respond to other cars and advisory signs has been developed and is presented in Chapter 3. Flow disturbances have been programmed, and the simulation run both with and without sign control in order to test the effectiveness of the sign control algorithm in reducing driver acceleration noise.

The advisory speed sign concept for expressways bears an analogy to the traffic signal funnel system^{19,24-27}. The signal funnel consists of a sequence of changeable speed signs in advance of a signalized intersection or crossing. When the traffic signal turns red the upstream signs advise motorists to decrease their speed, thus delaying their arrival at the intersection until the light turns green. The purpose of the signal funnel is to prevent delays due to poor starting performance of a long queue at the signal light. Subsidiary benefits are smoother velocity profiles and hence greater motorist comfort.

The concept of a remotely controlled speed sign on highways is not innovative. Several facilities have such signs that are used to change the posted speed limit when weather conditions warrant. However, a system of advisory speed signs being dynamically regulated by a computer in response to current local traffic perturbations has not been seriously proposed. Recent literature^{20,21}, however, indicates that thinking is progressing in the direction of on-freeway real-time responsive traffic control systems.

The surveillance and control system on the John C. Lodge Freeway¹⁵ included a limited study of the effects of changeable speed signs during peak periods. The results were inconclusive due to the following shortcomings of the study: (1) The motorists did not understand the advance warning nature of the signs and probably interpreted the speeds as regulatory, (2) the signs were not dynamically controlled; they were used in an attempt to reduce the flow upstream from fixed geometric bottlenecks, and (3) the signs were controlled by an operator on the basis of television surveillance rather than by computer processing of detector data. In addition, the freeway demand was not restricted by ramp control during peak periods.

It is not expected that advisory sign control will be effective when demand exceeds capacity. Also, light traffic poses no problems and does not warrant sign control. Advisory sign control is intended for conditions of dense traffic with the demand restricted to being below the capacity flow rate. Under such conditions congestion is often precipitated by a minor perturbation within a local pocket of high flow such as a compact platoon of several vehicles. It is expected that in a practical system ramp control and advisory sign control will be used together with the former maintaining the environment in which the latter can be effective.

In Chapters 2, 3, and 4 details of the sign control system are presented. In Chapters 2 and 3 the simulation program and the models used for vehicle dynamics are described. Chapter 4 presents a description and justification of the sign control algorithm. The numerical results comparing the system performance with sign control to the performance without sign control are included in Chapter 5.

CHAPTER 2

SIMULATION PROGRAM

2.1 Justification for Simulation

Positive proof of the effectiveness of the proposed control system can be obtained only by building a prototype on an existing highway and comparing before and after performance indices. However the high cost of building such a system mandates that a preliminary determination be made of the possible improvements afforded by the system. To make this determination a digital computer simulation program has been developed which incorporates the roadbed detectors, the advisory speed signs, the sign control algorithm, and the several models that govern the dynamics of vehicles as they respond to other cars and to the advisory signs. For a particular case the simulation is run both with and without advisory sign control and the performance indices are compared to determine the degree of improvement.

Such a simulation of a complex system involving the human element has the disadvantage of being both an approximation and an oversimplification of the actual dynamics. However it has the potential of being far more realistic than those analytic models that describe a driver's response by a single second order differential equation. Although an experiment on an actual highway is more realistic the simulation offers several significant advantages. For example, a physical modification, such as the spacing of the advisory signs, can be effected by a programming change in

a simulated system whereas the same change in an actual system would be expensive, time consuming, and disruptive. The sensitivity of the performance index to any parameter of the control system can be easily obtained since the inputs to the simulation are readily reproduced. A simulated highway also allows the effects of increased volume to be estimated before the actual demand exists.

2.2 The Simulation Program

Simulation Approach The simulation program is written in Fortran for the IBM 360/50 computer; it occupies about 112 K bytes of storage. Fortran was selected instead of one of the general purpose simulation languages such as GPSS and CSMP because of its greater flexibility and the faster execution time possible by using the Fortran-H compiler. The ratio of simulated time (vehicle-minutes) to CPU time is approximately 100.

On an actual highway every aspect of a vehicle's response to the motion of the car ahead or to an advisory sign varies among different drivers and varies in time for a particular driver. This simulation ignores the time variation and considers only two of the more critical parameters as random variables. One of these parameters is the desired headway factor for the i -th driver, $HDRV_i$, which also determines his sensitivity of response to the car ahead. The other is the compliance factor for the i -th driver, $COMPLY_i$, which specifies whether or not a driver heeds the advisory sign settings.

Since drivers have certain characteristics specified as samples from a probability density function it is advisable to repeat the simulation with different sample populations. It can then be ascertained to what extent different sample populations affect the improvements afforded by the control system and an expected value for the improvement can be estimated. Provision has been made to automatically repeat a simulation run using a different driver sample. The parameter NENS specifies the number of times

a simulation should be repeated under identical conditions except for different driver populations. The expected values of the headway factor and the compliance factor are held constant for these runs.

The Simulated System A single-lane, straight, level roadway is simulated on which no passing is allowed. Although it is not realistic to consider that expressways will have only one lane in urban areas, it is likely that a control strategy formulated for a single-lane roadway may be applied to a multi-lane roadway by the introduction of appropriate averages over the several lanes. A justification of control on the basis of a lane composite is that in heavy traffic the flow on all lanes of a particular section of roadway exhibits a certain uniformity. Thus it is assumed that the simulated lane of traffic represents one lane of a multi-lane expressway on which passing opportunities are rare due to dense traffic conditions. The high density assumption implies that every vehicle is coupled to the preceding vehicle; each vehicle responds, after a reaction time, to a significant acceleration of the car ahead. The models of vehicle dynamics are appropriate to this situation.

The length of the roadway may be an integral number of miles up to a maximum of 10 miles in the current version of the program. The roadway is considered to be a sequence of 0.1 mile segments referred to as sections.

A pair of vehicle detectors located at each section boundary are simulated. These detectors produce outputs that allow calculation of the speed of the vehicle most recently passing each detector and the vehicle

count on each 528 foot section of roadway. In practice the detectors would be of the induction loop design which are readily available, economical, and easily installed.

Computer controlled advisory speed signs are also simulated. These signs are similarly located at the section boundaries spaced by 0.1 mile. Each sign is either on or off as the current traffic conditions warrant. A sign that is on displays an advisory speed which is limited to one of the standard 5 mi/hr increments.

Since the length of roadway being simulated is intended to be a segment of a much longer highway it is necessary to avoid the end effects associated with vehicles exiting from the simulated roadway. When vehicle k exits from the roadway vehicle $k+1$ is deprived of its "leader" and unless appropriate measures are taken a disturbance can propagate back upstream from the terminal point. To alleviate this problem a runoff section equal in length to 20 percent of the unextended roadway is provided.

Input Parameters Several different types of input constants allow flexibility in the simulation. Certain constants are parameters in the models for car-following. By changing these parameters the effect, for example, of reaction time on stability can be observed. Other input constants describe physical items of a simulation run such as the length of roadway, the number of vehicles generated, and whether or not the advisory signs are to be used. Certain constants determine the speed and headways of vehicles as they enter the highway. There is provision for generating drivers with

uniform car-following characteristics or with sensitivity factor chosen from a probability density function so as to yield a specified input flow. Constants specifying the desired input flow and certain characteristics of the density function are inputs to the simulation. In addition certain parameters specify the desired dynamics of the first car. It is the programmed maneuvers of the first car that initiate all car-following responses. The options available for the motion of the first car are described in a later section. Another constant specifies the percent of drivers that obey the advisory speed signs. Certain constants select printout during the course of the simulation that is useful for debugging, tracing the microscopic motion of the vehicles, or tracing the logic of the sign control algorithm.

Simulation Output Printout may be obtained during the course of the simulation for purposes of debugging or of studying the simulation on a microscopic level.

An option permits printing the speed of all cars on the roadway at one second intervals. This output is printed at intervals of 5 seconds of simulation time between prescribed limits.

For the first twenty cars there is the option of printing position, distance from the car ahead, speed, acceleration, and current mode of operation. The time interval between printing of these variables is specified as an input constant. This form of detailed printout proved useful in the initial phase of testing and refining the car-following models. It has further use in plotting spacing versus speed for a particular maneuver.

An option is included that allows tracing of the logic of the sign control algorithm. At every sign setting interval a graphic representation of the highway is printed. The number of cars on each section and the detector velocity indication at each section boundary is clearly shown. Then intermediate variables in the computation of each sign setting are printed. This is followed by the computed sign setting or the appropriate explanation of why the sign is not to be turned on. After this is printed for each sign the results of the rounding algorithm follow. The sign setting rounded to one of the 5 mi/hr increments is printed or the reason why the sign is turned off is indicated.

Every 30 seconds a table is printed containing the number of vehicles on each section, the velocity indication at each detector site, and the setting of each sign. These data are of the six previous sampling instants spaced by 5 seconds. This table presents a clear picture of the sign profile at a particular time and its evolution over successive 5 second intervals.

At the conclusion of each run additional results are printed. These outputs are described below.

The output of a particular run is identified by printing the values used for certain of the input parameters or options that may be changed between runs. This output indicates the length of highway and the number of vehicles simulated, the type of maneuver undergone by the first vehicle, the entering flow rate, whether entering vehicles are generated with uniform or random headway factors, and whether or not sign control is being used.

For each vehicle the output lists its travel time, stopped time, desired headway factor, speed and spacing both upon entering and leaving the highway, and its acceleration noise for the trip.

A table of $COMPLY_1$ is printed indicating for each driver whether or not he responds to the advisory sign settings or ignores them. The percent of drivers that comply is also printed.

The duty cycle for each advisory speed sign is also given. For the i -th sign this quantity is the ratio of the number of 5 second intervals during which the i -th sign is on to the number of 5 second intervals at the beginning of which the i -th section is non-empty. The sum of these numerators divided by the sum of these denominators is an indicator of the overall duty cycle of the signs.

Cumulative statistics are printed giving, for all cars, the total travel time, total acceleration noise (not including that of the pre-programmed lead vehicle), and the total stopped time. Average statistics give the average travel time, average speed upon leaving the highway, average spacing upon entering and leaving the highway, and average acceleration noise (not including the lead vehicle).

When several runs are made for the purpose of obtaining results for different driver sample populations the ensemble statistics are printed at the end of the set of runs.

Storage of Variables The basic time increment used in the simulation is 0.2 seconds. In this interval of time a vehicle travels approximately one car length at 60 mi/hr. This time increment allows reaction time to be easily simulated to an accuracy of 0.2 seconds.

Since the car-following and sign-following models include time delays, it is necessary to retain certain values computed at previous time increments. Specifically, the acceleration of the k -th vehicle at time t is a function of the position and speed of it and of vehicle $(k-1)$ during the previous 2.0 seconds. Thus at any time t , 11 storage locations are needed for $x_k(t), x_k(t-0.2), \dots, x_k(t-2.0)$ and 11 locations are needed for $v_k(t), v_k(t-0.2), \dots, v_k(t-2.0)$. Sufficient storage is reserved for 350 vehicles in the current version of the program.

The usual approach for storing these variables is to employ double-subscript arrays. Then $X(I,J)$ might represent the position of the I -th vehicle at time $t=0.2(J-1)$ where t is the current simulation time and J is in the range $1 \leq J \leq 11$.

The approach described above is inefficient for two reasons. Firstly, at every update time ten move operations are necessary for each vehicle on the highway. Secondly, the indexing procedure for double-subscript arrays takes far more execution time than that for single-subscript arrays. The disadvantages are overcome by the following storage method.

The particular scheme used for storing the position and speed information is chosen to minimize the number of times that data is moved from one location to another. Figure 2-1 indicates how the values of position for 100 vehicles are stored. The rows correspond to storage locations and the column headings indicate the values of simulation time for which the contents of storage are shown. In row 2, column 3, the notation $x_1(65.0)$ indicates that at $t = 65.4$ sec, location 2 contains the value of position for car 1 at $t = 65.0$.

When t is incremented from $t = 65.0$ to $t = 65.2$ the new values for position are computed for all 100 cars. That is $x_i(65.2)$ are computed for $1 < i < 100$. As shown in column 2, the new value $x_i(65.2)$ is stored adjacent to the previous value $x_i(65.0)$. The quantity $x_i(65.2)$ replaces in storage the oldest value of position stored for the $(i-1)$ -th vehicle, $x_{i-1}(63.0)$, which is no longer needed. The arrows in Fig. 2-1 indicate where the most recent values are stored. At $t = 65.4$ it is noted that the new value $x_1(65.4)$ is stored at the bottom of the array since the top of the array has been filled. A single variable keeps track of the current position of car 1 (or the car furthest downstream if car 1 has already left the highway). The current positions of successive cars are spaced by 11 locations.

An identical storage arrangement is used for the speeds of the vehicles being simulated. This storage scheme obviates the need to shift values in storage or use double subscript notation. The one-dimensional arrays that store position and speed are designated by XCAR and VCAR

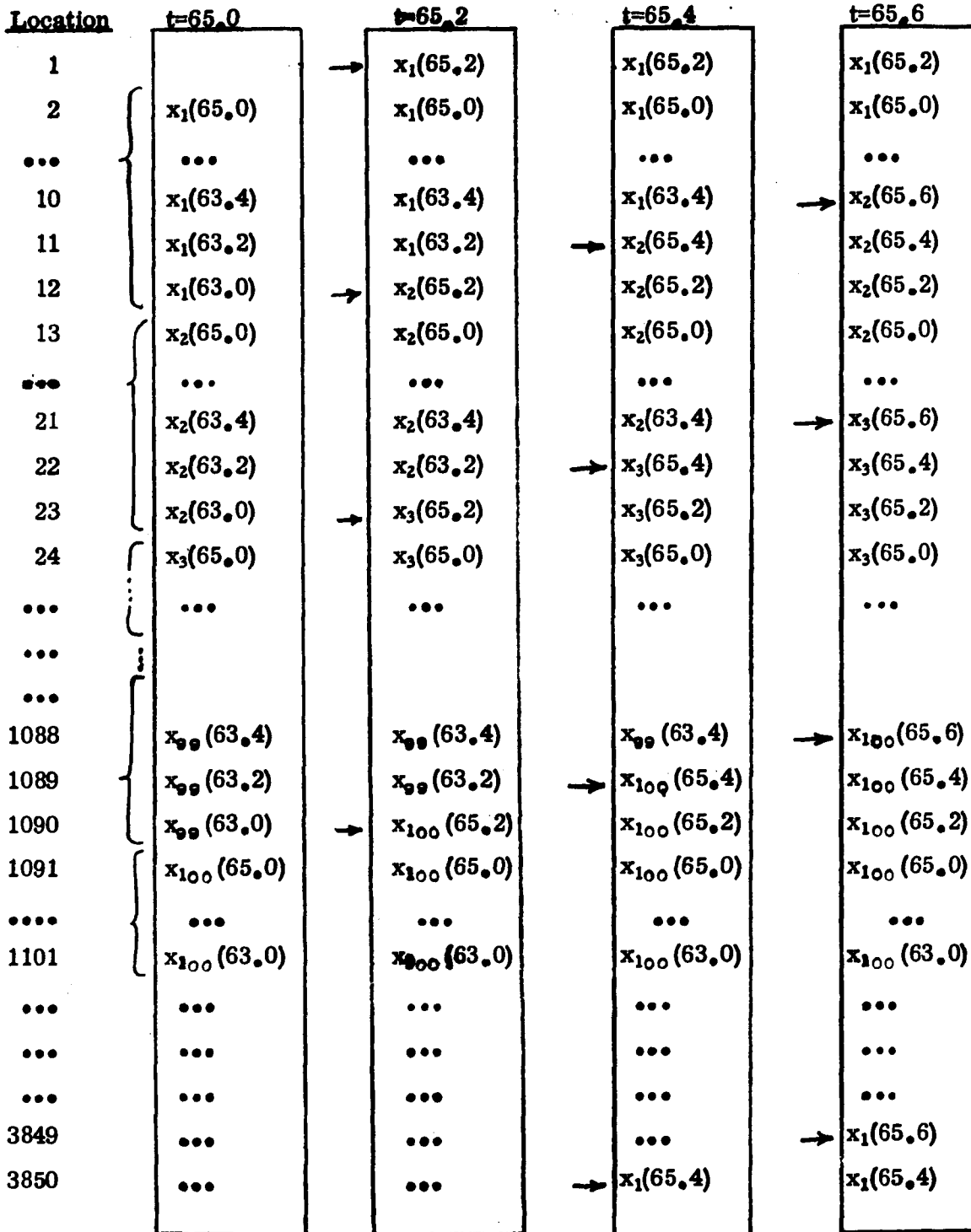


Fig. 2-1 Storage Layout

respectively in the Fortran program.

The current acceleration of the i -th vehicle is stored as $AN\emptyset W(I)$. The array $IBRNCH(I)$ stores the mode of each car at the previous simulation time which is used to determine the point of entry into the subroutine that computes each car's motion. That is $IBRNCH(I)$, in part, remembers the previous state of the I -th vehicle. The array $IDM\emptyset DE(I)$ stores another component of each vehicle's state, indicating the type of braking behavior the car was engaged in at the previous simulation time. The number of the highway section currently occupied by the I -th vehicle is stored in $CECT(I)$.

Detector derived data are stored in the arrays $ND(I,J)$ and $VD(I,J)$ with ND holding the number of vehicles on each section and VD holding detector speed indications. As the simulation time progresses the index J increases from $J = 2$ to $J = 7$ and then repeats this cycle being incremented at 5 second intervals. If at a particular time, say $t = 84$ sec., the second subscript has the value $J = 3$ then $ND(I,3)$ is the number of vehicles currently on section I . $ND(I,2)$ equals the number of vehicles that were on section I at the end of the previous 5 second interval, that is, at $t = 80$ sec. The quantities $ND(I,1)$, $ND(I,7)$, $ND(I,6)$, $ND(I,5)$, $ND(I,4)$ respectively represent the number of vehicles on section I at times 75, 70, 65, 60, and 55 seconds. In the same manner $VD(I,3)$ represents the detected speed from the vehicle most recently having passed detector I . And $VD(I,6)$ represents the speed detected at the I -th detector just prior to $t = 65$ sec.

The speed setting of the I-th advisory sign is stored in $VSIGN(I, J)$ and the array $LSIGN(I)$ indicates, for each sign, whether it is currently on or off. The second index J in $VSIGN(I, J)$ is utilized in the same manner as with ND and VD . In this way values of ND , VD , and $VSIGN$ at the six previous 5 second intervals are retained.

Generation of Vehicles The cars enter the roadway at a constant speed, V , which is the same for all vehicles and is an input parameter to the program. The vehicles continue at constant speed until all cars have entered the roadway, at which time the lead vehicle commences its maneuver. This method of entering the cars onto the highway assures that the initial conditions will be the same for a run without sign control as for a run with sign control, as explained further in Section 3.2.

The time of entrance of a vehicle is such that the desired following distance for the current speed exists between the vehicle and the vehicle ahead. The desired following distance for the i -th driver is a function of $HDRV_i$, the headway factor assigned to the i -th driver. By specifying the mean value of the headway factor, in addition to the entering speed, the average rate at which vehicles enter the roadway is determined, as is shown in Section 3.2. This rate at which vehicles are generated represents the demand for service on the roadway and is referred to by the letter D . The demand, D , is an input parameter that is varied between 1600 and 2300 vehicles per hour. After the lead

vehicle begins its maneuver, actual flow rates achieved on the simulated highway are between 1400 and 1700 vehicles per hour.

It is to be noted that the demand rate D differs from the usual definition of demand in that the vehicles in this simulation have been generated under perturbation-free conditions. On an actual highway, flow rates achieved under conditions of interacting vehicles are considerably lower as is found to be the case in this simulation.

Motion of First Vehicle The motion of the first car to enter the highway is pre-programmed according to several input parameters. The acceleration of the first vehicle is determined by the input constants $VI3$, $NREP$; $TF3(1), \dots, TF3(5)$; $AF3(1), \dots, AF3(4)$. The car's initial speed is $VI3$ and its acceleration is zero until $t = TF3(1)$. At this time the car initiates a maneuver described as follows:

$$a_1(t) = \begin{cases} AF3(1) & \text{for } TF3(1) < t < TF3(2) \\ AF3(2) & \text{for } TF3(2) < t < TF3(3) \\ AF3(3) & \text{for } TF3(3) < t < TF3(4) \\ AF3(4) & \text{for } TF3(4) < t < TF3(5) \end{cases}$$

After $t = TF3(5)$ this basic maneuver is repeated until it is executed $NREP$ times. Thereafter the car continues at constant velocity until it leaves the highway. The subroutine $F3$ controls the motion of the first car. It provides values for the current acceleration, the new velocity, and the new position of the first vehicle.

As shown above the motion of the first car is periodic with period $P = TF3(5) - TF3(1)$ seconds. The usual specification for the constants yields a constant deceleration followed by an interval of constant speed; then a constant acceleration back to the original speed followed by an interval of constant speed.

The first vehicle commences its maneuver shortly after all vehicles enter the highway and usually continues the periodic motion until the vehicle reaches the end of the highway extension (runoff section) where it is terminated.

Dynamics of Following Vehicles Vehicles respond to the motion of other cars and to the advisory speed signs according to the models given in subroutine G1. These models are described in detail in Section 3.1.

Advisory Sign Settings Subroutine SETSGN contains the logic for computing the speed settings of the advisory signs. Every 5 seconds sign settings are determined by analyzing the detector information in the arrays ND and VD. The sign settings are rounded to the standard 5 mi/hr increments. The details of the sign setting algorithm are presented in Chapter 4.

CHAPTER 3

MODELS OF VEHICLE DYNAMICS

3.1 Models of Vehicle Dynamics

Introduction There is a vast literature to draw upon in selecting mathematical models to describe how a vehicle responds to its environment, especially to the motion of other vehicles. These models, often termed car-following laws, are limited to highway traffic without passing, a situation that is approximated on multi-lane expressways with dense traffic.

Models of Single-lane Traffic -- Historical Background

Reuschel²⁸ (1950) and Pipes²⁹ (1953) were the first to study the motion of a line of cars under the assumption that vehicle spacing is linearly proportional to speed. The stimulus-response interpretation is that a vehicle's acceleration is proportional to its current relative velocity with respect to the car ahead.

Chandler, Herman, and Montroll³⁰ (1958) and Kometani and Sasaki^{31,32} (1958, 1959) were the first to allow for possible instability in car-following by introducing a reaction time into a linear car-following model. These researchers and others³³ analyzed the transient response of the following car and the propagation of a sinusoidal disturbance down a line of vehicles. Estimates for the parameters of these car-following laws were obtained from actual traffic data by statistical methods.

Lee³⁴ (1966) later generalized the previous linear models by expressing a vehicle's acceleration as a superposition integral involving a memory function multiplied by the relative velocity at previous times.

The first non-linear model for car-following was postulated by Gazis, Herman, and Potts³⁵ (1959) to provide a better curve-fit to experimental steady-state flow data. Additional non-linear models were proposed by several researchers³⁶⁻³⁹ in the following years and some limited investigations of the dynamics of traffic under non-linear models have appeared in the literature.⁴⁰⁻⁴²

Limitations of Basic Car-following Models

Essentially all of the above models are of the form

$$\ddot{x}_k(t) = \lambda_{m,n} \frac{\dot{x}_k^m(t)}{[x_{k-1}(t) - x_k(t)]^n} [\dot{x}_{k-1}(t-\tau) - \dot{x}_k(t-\tau)]$$

except for Lee's model given by

$$\ddot{x}_k(t) = \int_0^t M(t-\lambda) [\dot{x}_{k-1}(\lambda) - \dot{x}_k(\lambda)] d\lambda$$

The linear model, with $n=m=0$, is adequate in describing small fluctuations from a steady-state speed but is unrealistic when large changes in velocity and spacing occur. The non-linear models offer an extended range of applicability and, in addition, predict an increase in the average spacing between vehicles when the speed undergoes a periodic perturbation. However, the advantages of the above models due to their simplicity and mathematical tractability are counterbalanced by their limitations.

These models are limited in that τ and λ are fixed quantities. It has been found⁴³ that λ is different for acceleration and deceleration

maneuvers. The value of reaction time should also be different for acceleration and deceleration because of the safety consideration involved in a vehicle's response to a slowdown. Ideally the reaction time should be divided into components of perception time and time to effect the control response. Each of these component delays vary among different situations and different drivers and even depend on the severity of prior disturbances.

Another limitation is the failure of the above models to constrain vehicle accelerations to be within the range of vehicle performance.

These models are also limited to describing a vehicle's response as a velocity-sensitive mode whereas in reality the varied highway situations demand qualitatively different responses. Realistic modeling requires emphasis on understanding driver behavior to the extent that situations can be distinguished in which drivers react to different stimuli. It has been recognized that vehicles on a roadway are not always in the car-following mode where they respond to relative velocity. Michaels⁴⁴ (1963) analyzed the information used by the driver in three distinct modes: overtaking, steady-state following, and responding to an acceleration of the car ahead.

Formulation of the Simulation Models Most of the drawbacks of the simple car-following models can be overcome in a computer simulation because the constraint of mathematical tractability can be relaxed. Reaction time and sensitivity can be easily varied and can differ among drivers, thus introducing a realistic random aspect to traffic flow. Different modes of response characterized by different mathematical models can be easily implemented.

In this simulation a vehicle is considered, at a given time, to be in one of three principal modes: the car-following mode, the distance-detection mode, or the sign-following mode. In the car-following mode a vehicle is responding to the motion of the car ahead; in the distance-detection mode a vehicle is attempting to modify the spacing to the car ahead; and in the sign-following mode a vehicle is responding to the speed setting of an advisory sign.

The car-following models are non-linear but reduce to linear models for small perturbations from a steady-state. Deceleration and acceleration responses are computed by different models. The sign-following model is patterned after a linear car-following model with the constants appropriately modified. Variation among drivers is achieved by specifying a parameter appearing in the car-following model and in the distance-detection model as a random variable. Another random variable determines for each driver, whether or not he heeds the advisory sign settings.

The models governing a vehicle's dynamics, as specified by the logic of subroutine G1, are described in further detail in the following sections.

The Car-following Mode In general the i -th vehicle is undergoing car following if the $(i-1)$ -th vehicle is accelerating (or decelerating) at a significant rate or if the i -th vehicle is still responding significantly to a prior maneuver of the car ahead.

In order for the i -th vehicle to transfer from the car-following (CF) mode to the distance-detection (DD) mode it is required that: (a) the $(i-1)$ -th car have $|a_{i-1}| < 1.0$ for the previous 1.0 second and (b) the i -th vehicle have an acceleration within the range of that used in the DD mode and (c) that the rate of change of visual angle to the car ahead be sufficiently small.

To compute ACF, the acceleration due to car-following, it is first determined whether the response will be an acceleration or a deceleration since the model is different for each of these cases. In determining ACF for the i -th vehicle the expression $\Delta v(t-\tau) = v_{i-1}(t-\tau) - v_i(t-\tau)$ is evaluated for $\tau = \tau_a$ and $\tau = \tau_d$. These different values of τ represent the time lag associated with acceleration and deceleration. The values used are $\tau_a = 1.4$ sec. and $\tau_d = 1.0$ sec.

If $\Delta v(t-\tau_d)$ is negative then a deceleration is indicated. If both $\Delta v(t-\tau_d)$ and $\Delta v(t-\tau_a)$ are positive then an acceleration is indicated. If $\Delta v(t-\tau_d) > 0$ and $\Delta v(t-\tau_a) < 0$ then ACF is taken to be zero.

Car-following Deceleration Model For a deceleration the model for ACF is

$$ACF = \text{FACTOR} \times \Delta v(t-\tau_d) / \text{HDRV}_i \quad (3-1)$$

subject to modifications discussed below. The constant HDRV_i , termed the headway factor, is generated for each vehicle as an independent sample from a specified probability density function. The term FACTOR accounts

for the difference between the driver's desired spacing to the rear bumper of the car ahead, SD , and the actual spacing, SA . These spacings are given by

$$SD \equiv HDRV_i \times v_{i-1}(t)$$

$$SA \equiv x_{i-1}(t) - x_i(t) - 20$$

Using this notation, FACTOR is expressed as

$$FACTOR = \begin{cases} 1.0 & \text{for } v_{i-1}(t) < 20 \\ \frac{20}{v_{i-1}(t)} + \left[1 - \frac{20}{v_{i-1}(t)}\right] \left[\frac{SD}{SA}\right] & \text{for } v_{i-1}(t) \geq 20 \end{cases}$$

The quantity FACTOR biases the deceleration according to whether the inter-vehicle spacing is larger or smaller than desired. FACTOR varies from 1.0 to $\frac{SD}{SA}$ as $v_{i-1}(t)$ increases from zero toward infinity. Also, when $SD = SA$ it is noted that $FACTOR = 1.0$.

The deceleration of vehicle i is modified in certain cases if the acceleration of the car ahead is negative. If SA/SD is less than 1.6, the value of ACF given by Eq.(3-1) is modified by addition of the quantity

$$FCTR \times a_{i-1}(t) = \sqrt{\frac{1.6 - SA/SD}{9.6}} a_{i-1}(t)$$

FCTR varies from 0.338 when $SA/SD = 0.5$ to 0.25 when $SA = SD$ to zero when $SA/SD = 1.6$. This term accounts for an anticipatory reaction due to car (i-2) since $a_{i-1}(t)$ is proportional to $v_{i-2}(t-\tau) - v_{i-1}(t-\tau)$. The term FCTR is plotted in Fig. 3-1.

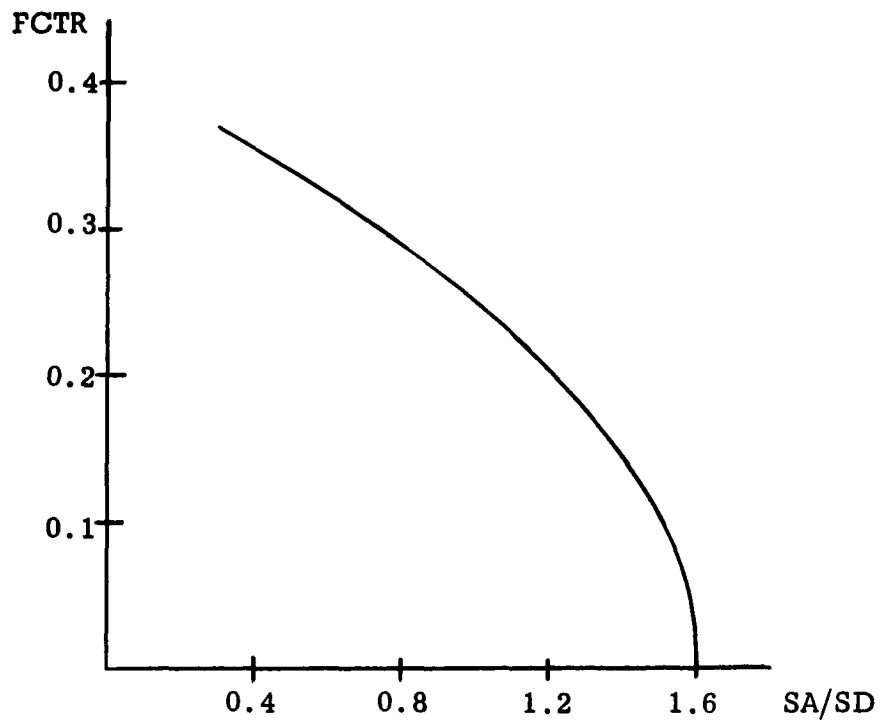


Fig.3-1 Anticipatory factor used in deceleration model

The magnitude of decelerations are limited to be less than 10 ft/sec² unless an unsafe spacing condition exists or unless the vehicle is still responding to a prior unsafe spacing situation. The minimum safe spacing is given by

$$SSAFE = \frac{1}{2a_m} \{v_{i-1}^2(t) - v_i^2(t) + [a_m - a_i(t)][2v_i(t) + a_i(t)\tau]\tau\}$$

Then SSAFE equals the minimum inter-vehicle separation needed to avoid a collision if vehicle (i-1) suddenly decelerates to a standstill at the rate of $a_m = -24 \text{ ft/sec}^2$. Vehicle i is assumed to decelerate at $a_m = -24 \text{ ft/sec}^2$ after a minimum reaction time of $\tau = 0.45 \text{ sec}$. The small reaction time is appropriate since the driver's foot is already on the brake.

If the car ahead is accelerating and if $v_i(t)$ is less than $v_{i-1}(t)$, even though v_i was greater than v_{i-1} at $t - \tau_d$, then the magnitude of the previously computed deceleration is decreased by multiplying ACF by the quantity

$$FT = \begin{cases} 1 - \frac{a_{i-1}(t)}{4} & \text{for } 0 < a_{i-1}(t) < 4.0 \\ 0 & \text{for } a_{i-1}(t) \geq 4.0 \end{cases}$$

Car-following Acceleration Model

For an acceleration the model

for ACF is

$$ACF = \begin{cases} \Delta v(t - \tau_a) / HDRV_1 \times \left[1 - \frac{0.25}{HDRV_1} \times \frac{\Delta v(t - \tau_a)}{AMAX} \right] & \text{for } \Delta v(t - \tau_a) < 2 \times AMAX \times HDRV_1 \\ AMAX & \text{for } \Delta v(t - \tau_a) > 2 \times AMAX \times HDRV_1 \end{cases}$$

where $AMAX$ is the maximum acceleration of which vehicle 1 is capable and is given by

$$AMAX = 12.4 - 0.0913 v_1(t) \quad \text{ft/sec}^2$$

The value of ACF may be increased, to a maximum of 2.0 ft/sec^2 , if the spacing to the car ahead is larger than normal. If SA/SD is greater than 1.2, the quantity FA is computed as

$$FA = \begin{cases} 10(SA/SD - 1.2) & \text{for } 1.2 < SA/SD < 1.4 \\ 2.0 & \text{for } SA/SD \geq 1.4 \end{cases}$$

If FA is greater than ACF then ACF is increased to FA.

If SA/SD is less than 1.6 and the car ahead is decelerating and if $v_1(t)$ is greater than $v_{i-1}(t)$, even though v_1 was less than v_{i-1} at $t - \tau_a$, then the previously computed acceleration is decreased (to a minimum of 0) by multiplying ACF by the quantity

$$FS = \begin{cases} 1 + \frac{a_{i-1}(t)}{4} & \text{for } -4 < a_{i-1}(t) < 0 \\ 0 & \text{for } a_{i-1}(t) \leq -4 \end{cases}$$

The Distance-detection Mode In the distance-detection, DD, mode a vehicle responds to the current spacing between his vehicle and the car ahead. If this spacing is reasonably close to the desired spacing then the vehicle continues at constant speed. Otherwise the vehicle initiates a maneuver to increase or decrease the spacing.

In general a vehicle is in the DD mode if the absolute magnitude of

the acceleration of the car ahead has been less than 1.0 ft/sec^2 during the previous second. A vehicle transfers from the DD mode to the CF mode when the above condition is violated and the acceleration due to car-following is outside the range of that possible in the DD sub-mode that the vehicle was in.

The DD mode is divided into sub-modes 1,2,3,4, and 5. The i -th vehicle is in mode 1 if its actual spacing, DA, and desired spacing, DD, to the car ahead at the current time satisfy

$$0.85 + 1/[v_{i-1}(t) + 10] < DA/DD < 1.2$$

where

$$DA = SA + 20 = x_{i-1}(t) - x_i(t)$$

$$DD = SD + 20 = HDRV_i \times v_{i-1}(t) + 20$$

In this mode the vehicle's acceleration is zero.

A vehicle transfers to mode 2 if it begins following too close to the car ahead. In mode 2 the vehicle decelerates according to

$$a_i(t) = \begin{cases} 0.8(-.03114 v_i(t) - 0.390) & \text{for } v_i(t) > 27.63 \\ -1.0 & \text{for } v_i(t) \leq 27.63 \end{cases} \quad (3-2)$$

This value of $a_i(t)$ approximately equals eighty percent of the deceleration obtained by removing the foot from the accelerator without applying the brake. In mode 2 the vehicle decreases its speed so that the spacing to the car ahead increases until the condition

$$DD - DA \leq \frac{1}{4} [v_i(t) - v_{i-1}(t)]^2 \quad (3-3)$$

is satisfied. At this time the vehicle transfers to mode 3. In mode 3 the i -th vehicle accelerates at 2.0 ft/sec^2 , while the inter-vehicle spacing is still increasing, until $v_i(t) \approx v_{i-1}(t)$. At this time, due to the condition of Eq.(3-3), the spacing is such that $DA \approx DD$ and the vehicle transfers to mode 1.

A vehicle is in mode 4 if it is too far from the car ahead. In this mode a vehicle begins to close the gap by accelerating at 2.0 ft/sec . with the restriction that its speed be not more than 12.0 ft/sec . greater than that of the vehicle ahead. At the appropriate time the vehicle transfers to mode 5. In mode 5 the vehicle decelerates by eighty percent of full engine braking, according to Eq.(3-2), until $v_i(t) \approx v_{i-1}(t)$. The point of switching from mode 4 to mode 5 is chosen such that $DA \approx DD$ when $v_i \approx v_{i-1}$. The vehicle next transfers to mode 1.

The Headway Factor The headway factor $HDRV_1$ appears in the models of both the car-following and the distance-detection modes. The correspondence between the dynamic car-following law and the steady-state speed-spacing relationship is now shown.

For small perturbations from a steady-state speed the car-following laws for both decelerations and accelerations reduce to the linear form

$$ACF = a_i(t) = \Delta v(t-\tau)/HDRV_1 = [v_{i-1}(t-\tau) - v_i(t-\tau)]/HDRV_1 \quad (3-4)$$

Assume that vehicle $i-1$ is initially at constant speed and undergoes a small change in speed to a new steady-state value. Vehicle i responds

according to Eq.(3-4) and eventually attains the same steady-state speed as vehicle $i-1$. The change in separation between these vehicles for this transition is found by integrating Eq.(3-4) to obtain

$$\Delta S = \text{HDRV}_i \times \Delta v$$

where S is the separation between front bumpers of vehicle $i-1$ and vehicle i . This result is consistent with the steady-state speed-spacing law given by

$$DD = \text{HDRV}_i \times v_{i-1} + 20$$

where DD is the desired steady-state separation.

Thus, for small perturbations, the car-following law that determines a vehicle's response also assures that the final spacing is correct for the new steady-state velocity. For larger perturbations the car-following laws become non-linear and result in a larger change in spacing than is desired. In these cases, after completing the car-following response, the vehicle transfers to the distance-detection mode to adjust its steady-state spacing.

The fact that HDRV may be a random variable allows for variation in dynamic response and steady-state following among drivers. The average value of HDRV determines the rate at which vehicles are generated as explained in Sections 2.2 and 3.2.

Sign Following After a vehicle's acceleration has been computed in either the car-following or the distance-detection mode,

control is transferred to the sign-following logic. If advisory sign control is being used the vehicle's acceleration due to a sign, *ASIGN*, is then determined. The mathematical model for the response to a sign setting is presented in Chapter 4.

To determine the effect of varying the levels of driver compliance on the performance improvement due to sign control, the random variable *COMPLY* is incorporated into the simulation. For each vehicle a sample of the random variable is generated, having the value zero or one, which determines whether a driver follows or ignores the sign settings. The expected value of *COMPLY* is *PCMPLY* which is the fraction of drivers that respond to the signs. *PCMPLY* is an input parameter to the program.

Overall Acceleration In accord with the philosophy that the signs are used to warn of impending slowdowns rather than to attempt to speed up traffic, the overall acceleration at a specific time is given by

$$a_i(t) = \min [ASIGN, A]$$

where *A* represents the acceleration in either the CF mode or DD mode as is appropriate.

3.2 Generation of Headway Factors

The primary goal of the simulation is to compare the vehicles' performance under advisory sign control to that without control. Therefore a computer run with control must start with the same initial configuration of vehicles as its counterpart without sign control.

This requirement implies that all vehicles must be on the highway before the start of the lead vehicle's maneuver. Otherwise the results with and without sign control would not be directly comparable since when a disturbance propagates back to the highway entrance some vehicles may be delayed in entering the roadway. The delay with sign control is different from the delay without sign control because of the different manner in which flow disturbances propagate.

Thus it is assumed that all vehicles enter the highway at the same speed V and proceed at constant speed until all cars are on the roadway. A vehicle's spacing, or time gap, upon entering the highway is equal to the driver's desired spacing for the entering speed V . The headway factor H , which determines the desired spacing and the sensitivity to car-following maneuvers, is the random variable that differentiates among drivers.

By specifying the entering speed and the mean value of the headway factor, the average rate at which vehicles enter the roadway is determined, as explained below. The discussion in this section relates the rate, D , at which vehicles enter the highway, called the demand, to $H\bar{B}AR$, the

average value of the headway factor.

It is emphasized that D represents the rate at which vehicles are generated at the entrance under perturbation-free conditions. After the lead vehicle begins its maneuver the following vehicles can no longer maintain their desired spacings and the resulting flow rates are in the range of 1400 to 1700 vehicles per hour, considerably below D .

Under the assumed conditions for entering vehicles the spacing (front bumper to front bumper) between the $(i-1)$ -th and the i -th vehicle is given by $d_i = HDRV_i \times V + 20$. In this expression $HDRV_i$ is a sample of a random variable H with probability density function $f(h)$. The particular form of this density function and the manner in which its shape depends on D is discussed in detail below.

The time headway at the entrance between vehicle $i-1$ and vehicle i is $t_i = d_i/V = HDRV_i + 20/V$. Since V is the same for all entering cars the average time headway at the entrance is $\bar{T} = HBAR + 20/V$ where $HBAR$ is the expected value of the headway factor. The average flow at this point is $D = 1/\bar{T} = 1/(HBAR + 20/V)$. To yield a specified value of D requires that the entering headways satisfy $HBAR = 1/D - 20/V$ where D is in units of sec^{-1} (i.e. vehicles per second). It is noted that since the size of the gap between cars is $H \times V$ the time headway of the gap is $T_g = H$ and thus T_g and H have the same distribution.

The density function chosen for H is shown in Fig.3-2. The values of H_{MIN} and H_{MODE} are fixed given quantities. H_{MIN} is the smallest headway factor allowed or the minimum time gap between the rear bumper of the car ahead and the front bumper of the following car. H_{MODE} is the most likely value of the headway factor. The values $H_{MIN} = 0.3$ and $H_{MODE} = 1.0$ are taken to be reasonable. The maximum headway factor is a variable which is determined once D is specified.

This approach is justifiable for the following reasons. At any value of flow or speed there will always be a percentage of drivers who follow with a minimum time gap, taken here to be 0.3 sec. Suppose that for a fixed speed V the headway factor density has mode = H_{MODE} and $\max h = H_{MAX_2}$ (see Fig.3-2) which determines D_2 . To have a larger demand D_1 for the same entering speed requires a larger average density (smaller H_{BAR}). The flow with smaller H_{BAR} will be characterized by the elimination of the large gaps. From other researchers' work⁴⁵ it is known that the mean headway of close following vehicles remains essentially constant as flow changes. By keeping the mode fixed the mean time gap of those vehicles with $h < H_{MODE}$ will remain constant as D changes.

This is shown by considering the conditional headway distribution $F_H(y)$ defined by

$$F_Y(y) = \Pr[H \leq y/H < H_{MODE}] \quad (3-5)$$

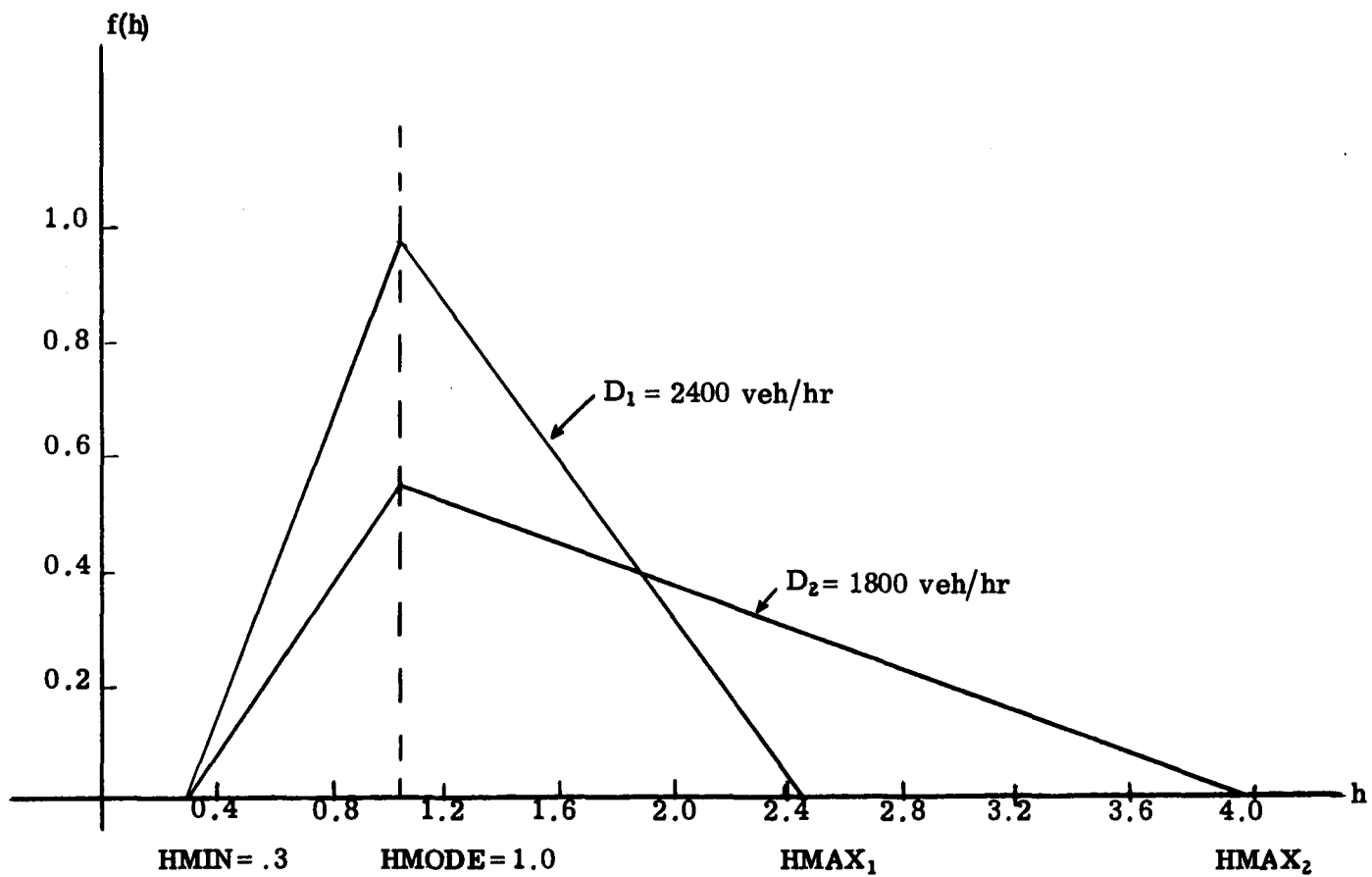


Fig.3-2 Headway factor density function

The $E\{Y\}$ is given by

$$E\{Y\} = HMIN + \frac{2}{3} [HMODE - HMIN] \quad (3-6)$$

which is independent of HMAX and D . Substituting the assumed values

HMIN = 0.3 and HMODE = 1.0 in Eq.(3-6) gives

$$E\{Y\} = 0.767 \text{ sec.}$$

as the expected headway factor or time gap for those vehicles with headways less than the modal headway. For these vehicles the expected time headway (front bumper to front bumper) is

$$E\left\{Y + \frac{20}{V}\right\} = 0.767 + \frac{20}{V} \text{ sec.}$$

The headway factor distribution function is shown in Fig.3-3. From the density function, the mean value of the headway factor or time gap is computed as

$$HBAR = \frac{1}{3} [HMIN + HMODE + HMAX] \quad (3-7)$$

Thus HMAX is determined by the assumed values for HMIN and HMODE and the desired value of HBAR to yield a specified D . From Eq.(3-7)

$$HMAX = 3 HBAR - HMIN - HMODE$$

where

$$HBAR = \frac{1}{D} - \frac{20}{V}$$

As an example, suppose the entering velocity is 50 mi/hr = 73.3 ft/sec. and the desired average flow input is $D = 1800$ vehicles/hr = 0.5 vehicles/sec. Then $HBAR = \frac{1}{0.5} - \frac{20}{73.3} = 2.0 - 0.273 = 1.73$ sec.

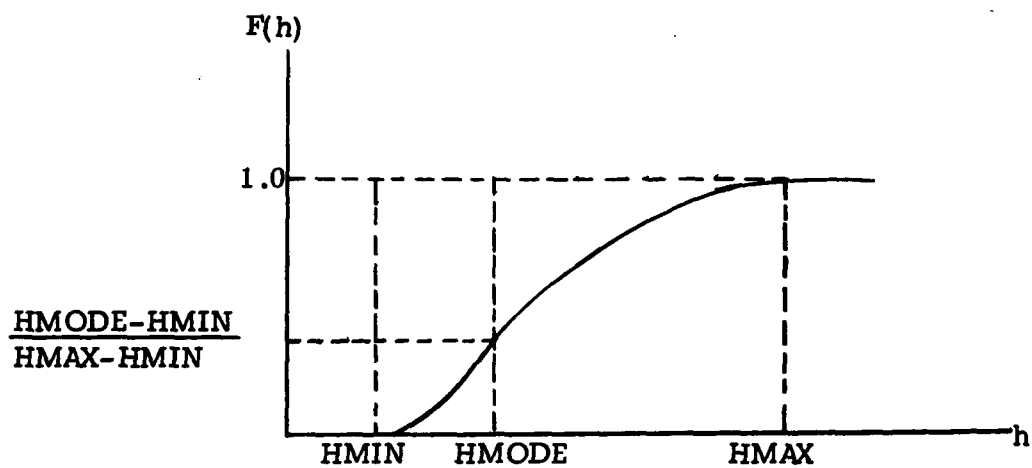


Fig.3-3 Headway factor distribution function

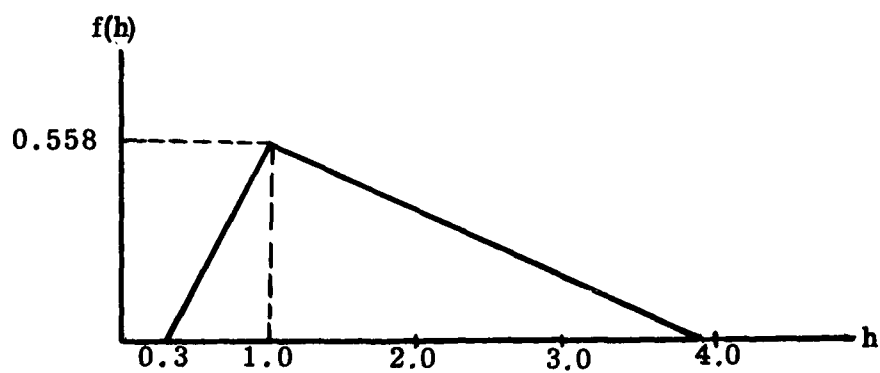


Fig.3-4 Headway factor density function for $D = 1800$ veh/hr,
 $V = 50$ mi/hr

and $H_{MAX} = 3(1.73) - 0.3 - 1.0 = 3.89$ sec. The average time headway is $\bar{T} = 1/D = 2.0$ sec. The density function for H is shown in Fig. 3-4.

The distribution function $F_H(h)$ is found by integration to be

$$F_H(h) = \begin{cases} \frac{(h - H_{MIN})^2}{(H_{MAX} - H_{MIN})(H_{MODE} - H_{MIN})} & \text{for } H_{MIN} \leq h \leq H_{MODE} \\ 1 - \frac{(H_{MAX} - h)^2}{(H_{MAX} - H_{MIN})(H_{MAX} - H_{MODE})} & \text{for } H_{MODE} \leq h \leq H_{MAX} \end{cases} \quad (3-8)$$

Evaluating $F_H(h)$ at $h = H_{MODE}$ gives

$$F(H_{MODE}) = \frac{H_{MODE} - H_{MIN}}{H_{MAX} - H_{MIN}} \quad (3-9)$$

From the above results $F_H(H_{MODE} = 1)$ can be plotted versus H_{MAX} . This is the probability that a headway factor (or gap) is less than the mode (assumed to be 1) versus the maximum headway factor. Using $H_{MIN} = 0.3$ and $H_{MODE} = 1$ Eq.(3-9) gives

$$F(H_{MODE} = 1) = \frac{0.7}{H_{MAX} - 0.3}$$

which is plotted in Fig. 3-5. Substituting for H_{MAX} in terms of D as given by

$$H_{MAX} = 3 H_{BAR} - H_{MIN} - H_{MODE} = \frac{3}{D} - \frac{60}{V} - 1.3$$

yields

$$F(H_{MODE} = 1) = \frac{0.7}{\frac{3}{D} - \frac{60}{V} - 1.6}$$

For $V = 50$ mi/hr = 73.3 ft/sec. this becomes

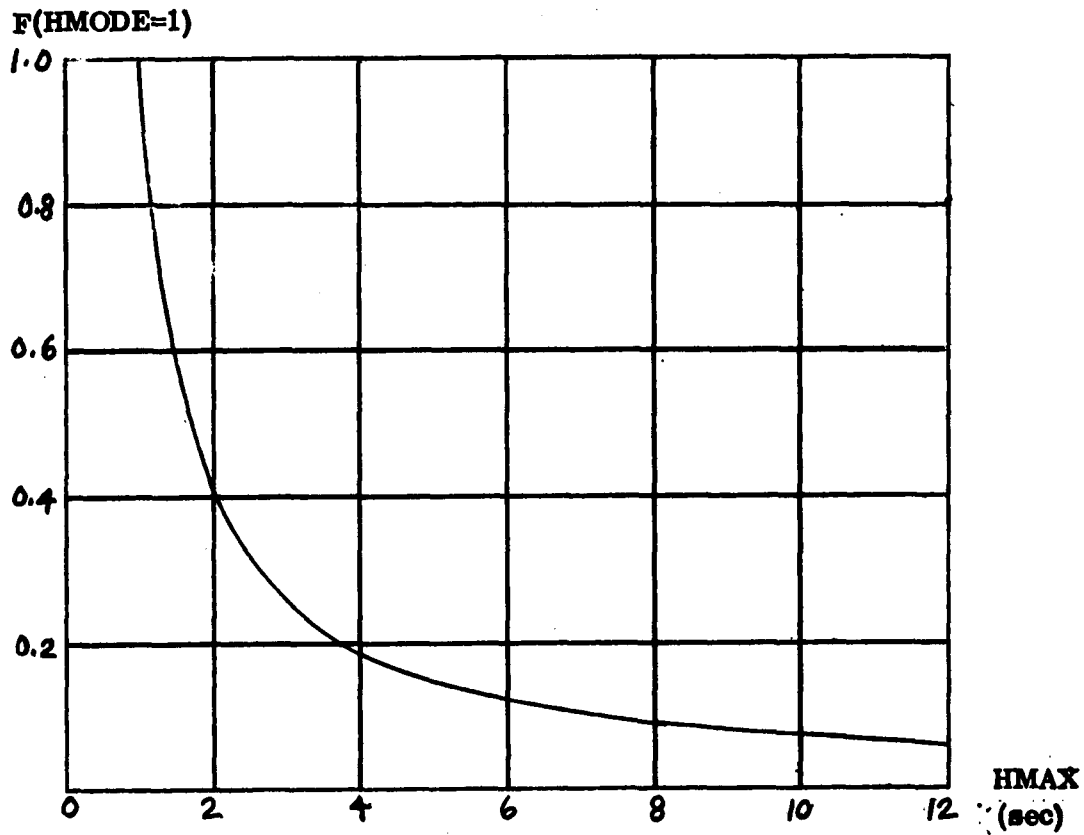


Fig.3-5 : Probability that Headway Factor is Less than 1 vs. HMAX

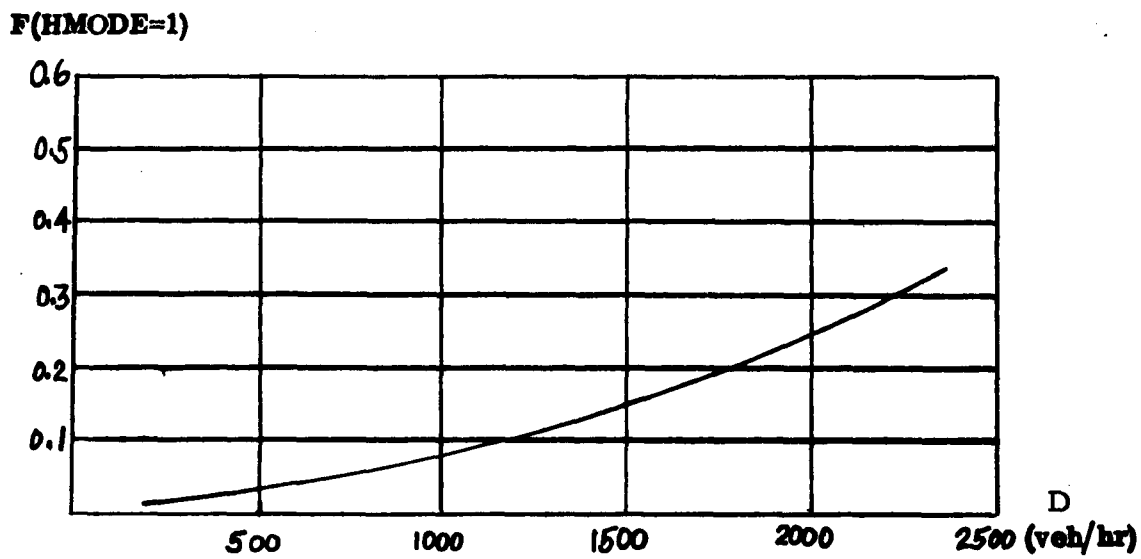


Fig.3-6: Probability that Headway Factor is Less than 1 vs. D
for $V = 50$ mi/hr

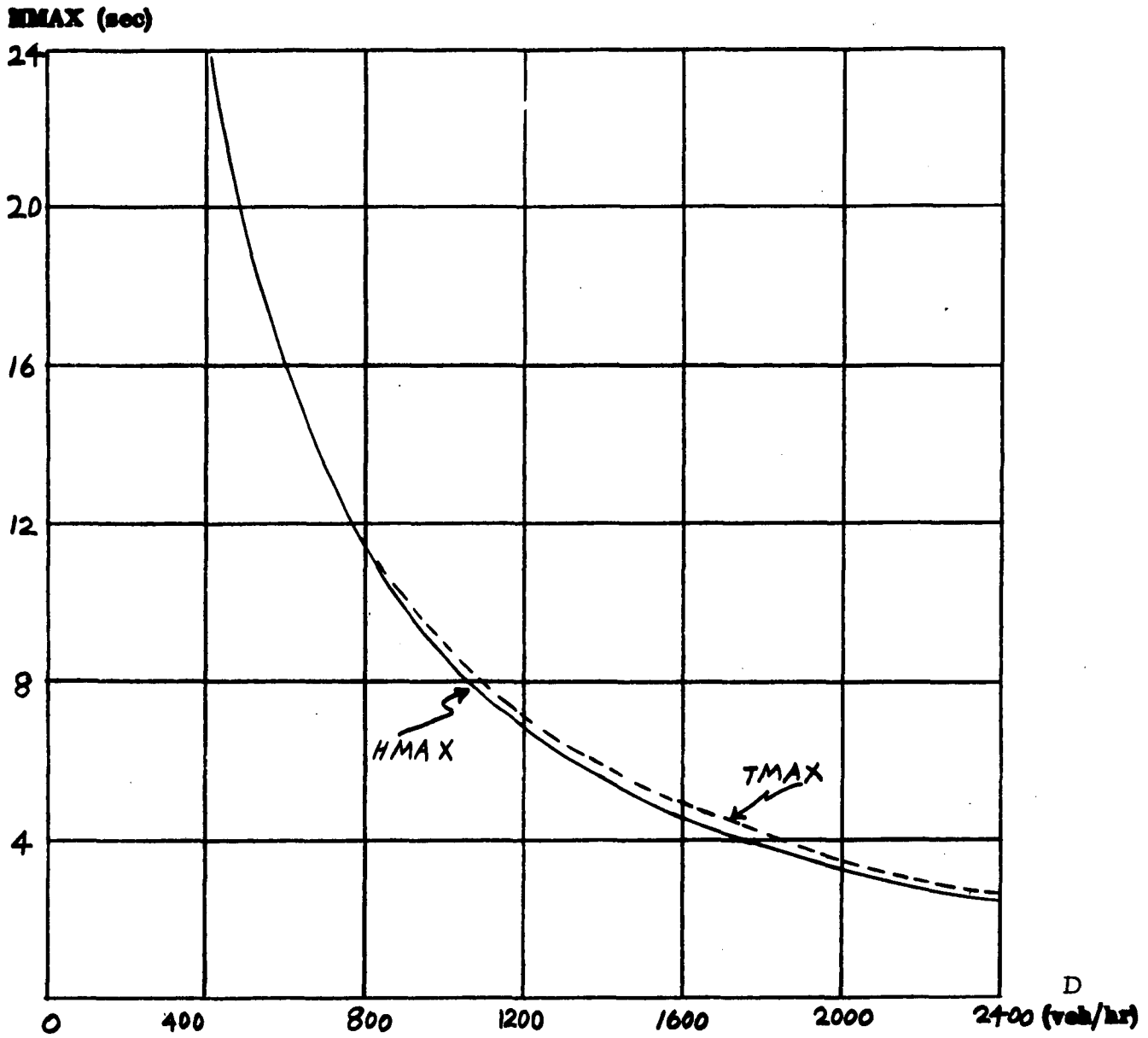


Fig.3-7: Maximum Headway Factor vs. D and Maximum Time Headway vs. D for V = 50 mi/hr

$$F(\text{HMODE} = 1) = \frac{0.7}{\frac{3}{D} - 2.42}$$

which is plotted in Fig. 3-6.

Figure 3-7 is a plot of the maximum headway factor (or gap) as a function of the demand rate for $V = 50$ mi/hr.

$$\begin{aligned} \text{HMAX} &= 3 \text{HBAR} - \text{HMIN} - \text{HMODE} \\ &= 3\left(\frac{1}{D} - \frac{20}{V}\right) - 1.3 = \frac{3}{D} - 2.12 \end{aligned}$$

Also shown on Fig. 3-7 is TMAX, the maximum headway (seconds) versus D (for $V = 50$ mi/hr), as given by

$$\text{TMAX} = \text{HMAX} + \frac{20}{V} = \text{HMAX} + 0.273$$

Data from Michigan freeways⁴⁶ agree well with our assumed density function. The Michigan studies found that approximately 2/3 of the vehicles are spaced at less than the mean headway for a wide range of average flow. From our density function

$$\begin{aligned} \Pr [T_g < \bar{t}] &= \Pr [H < \text{HBAR}] = F(\text{HBAR}) \\ &= 1 - \frac{(\text{HMAX} - \text{HBAR})^2}{(\text{HMAX} - \text{HMIN})(\text{HMAX} - \text{HMODE})} \end{aligned}$$

Substituting $\text{HMIN} = 0.3$, $\text{HMODE} = 1.0$ gives

$$\text{HBAR} = \frac{1}{D} - \frac{20}{V} = \frac{1}{D} - 0.273 \quad (\text{for } V = 50 \text{ mi/hr})$$

$$\text{HMAX} = 3 \text{HBAR} - \text{HMIN} - \text{HMODE} = \frac{3}{D} - \frac{60}{V} - 1.3 = \frac{3}{D} - 2.12$$

$$F(\text{HBAR}) = 1 - \frac{(\frac{2}{D} - 1.85)^2}{(\frac{3}{D} - 2.42)(\frac{3}{D} - 3.12)}$$

This is plotted in Fig. 3-8. It is seen that $F(\text{HBAR})$ is essentially constant at 0.55 over a wide range of demand rate. The fact that $F(\text{HBAR})$ is constant agrees with the Michigan studies and the actual value of 0.55 is not far from their range of 0.64 - 0.69.

The headway distributions for different 1 minute flow rates obtained in the Michigan study are shown in Fig. 3-9. The variation in the shape of the density function for different flow rates is similar to that of our proposed density function. It is noted that the Michigan study gives the headway density function whereas we give the headway factor (or gap) density function. A direct comparison is not possible since it is not known at what speed the measured flow levels were attained.

It is seen that the modal headway varies between 1.2 and 1.7 sec for lanes 2 and 3 for flows between 1000 and 2000 vehicles/hour. We have assumed $\text{HMODE} = 1$ and thus $t_{\text{mode}} = \text{HMODE} + 20/V$. Tabulating t_{mode} for a reasonable range of V gives

<u>V(ft/sec)</u>	<u>t_{mode}</u>
90	1.22
70	1.29
50	1.40
30	1.67

which shows a range of t_{mode} similar to the Michigan study.

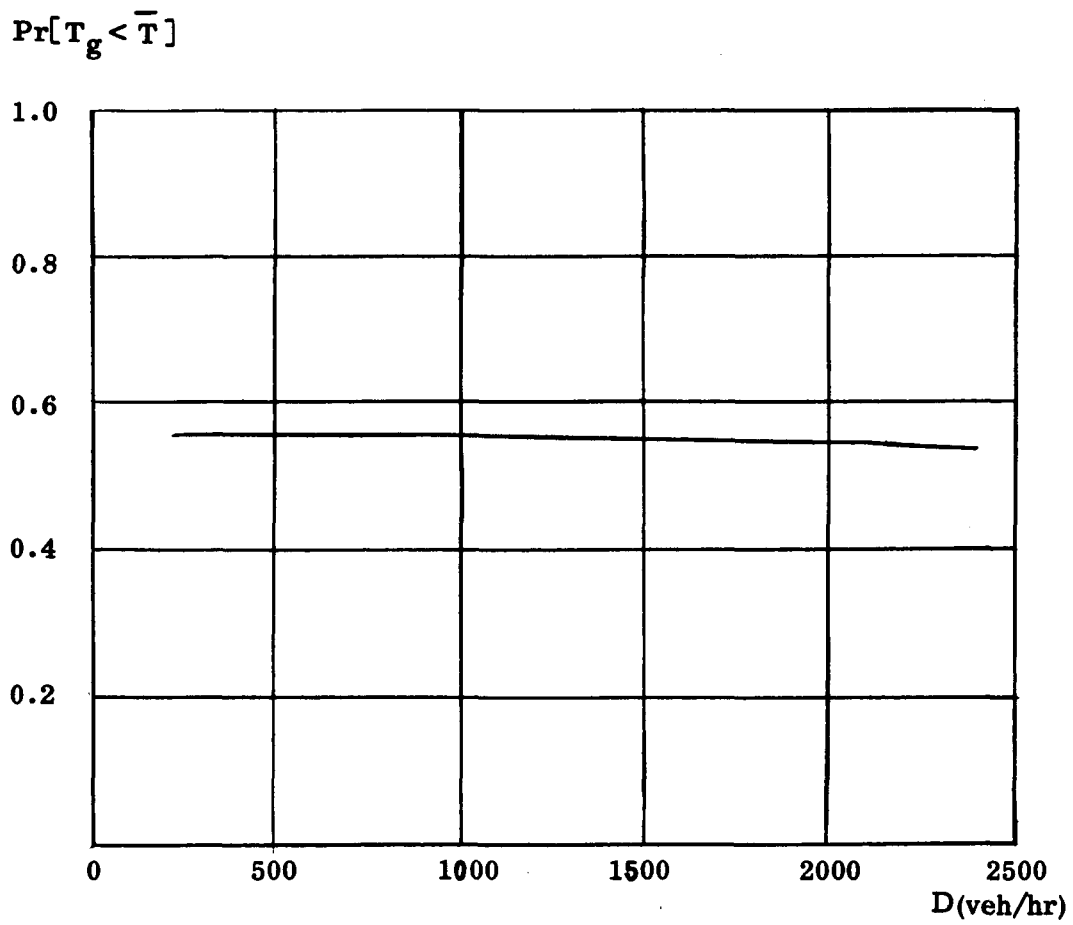


Fig.3-8 Probability that Headway is Less than Mean Headway vs. D for V = 50 mi/hr

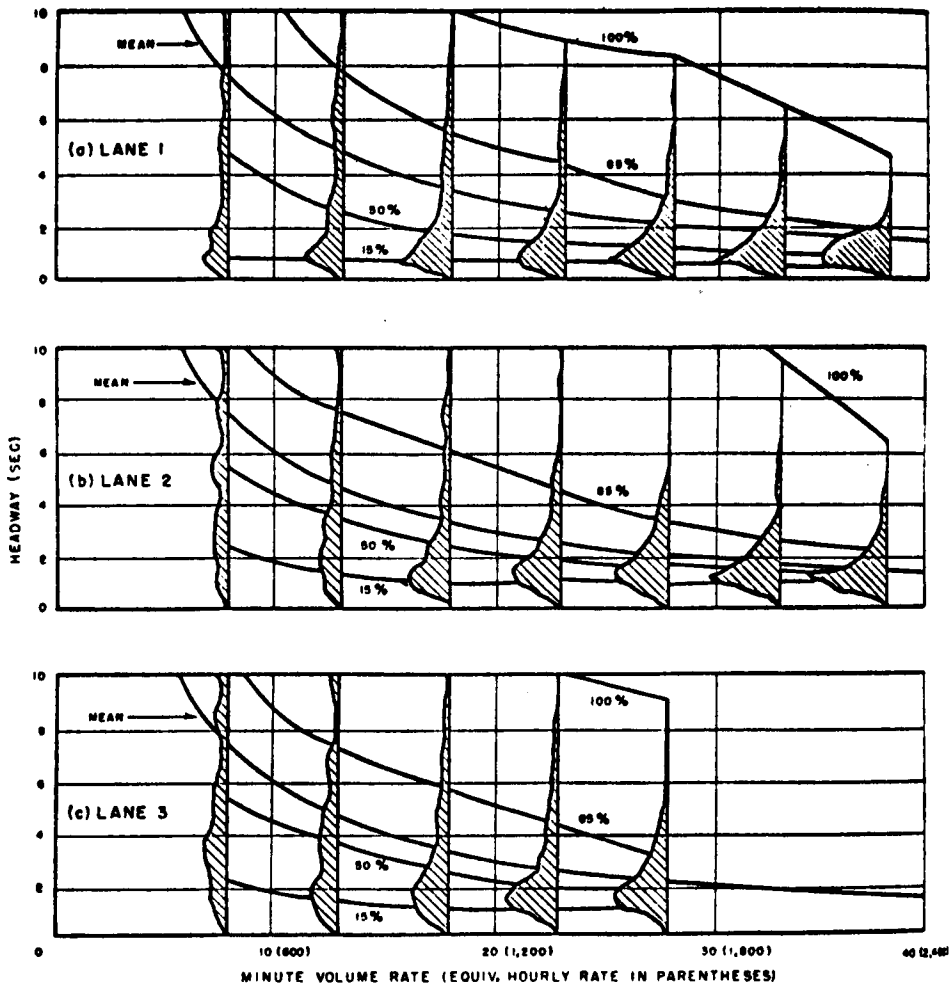


Fig. 3-9 Headway Density Function vs. D

Our density function also agrees with the Michigan study in that (1) the mode is always less than the median headway and (2) the median is less than the mean headway.

Thus, having made a good case for our proposed density function the procedure for generating entering headways is given below.

1. Choose appropriate values for entering velocity V and entering flow rate D .
2. Then compute $H\bar{B}AR = \frac{1}{D} - \frac{20}{V}$ and $HMAX = 3 \times H\bar{B}AR - HMIN - HMODE = 3 \times H\bar{B}AR - 1.3$. The density function for headway factor, $f(h)$, and $F(h)$, are thus completely determined.
3. Generate a pseudo-random number R in the interval $(0, 1)$.
4. Calculate $h = F^{-1}(R)$. Then $h = F^{-1}(R)$ is a sample from $f(h)$. [See Fig. 3-10].
5. Calculate $d = hV + 20$ as the distance headway for this vehicle at the entrance. Repeat steps 3, 4, 5 to generate successive vehicles.

It is noted that this density function is proposed to represent the headway factor (or gap) density on a typical lane of a multi-lane expressway under non-congested conditions.

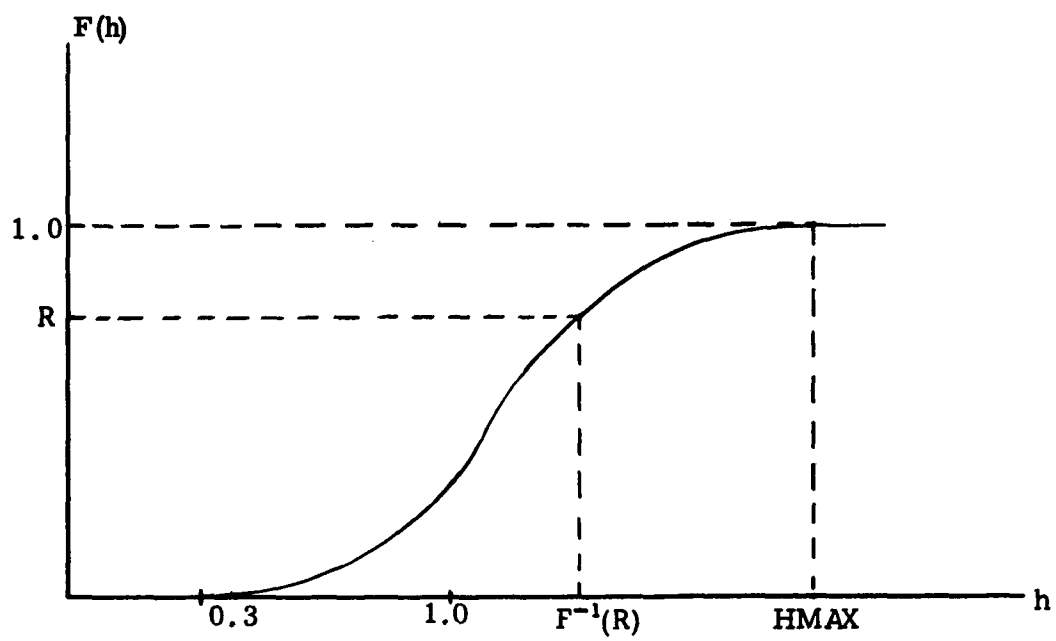


Fig.3-10 Generating samples from $f(h)$

CHAPTER 4

DERIVATION AND APPLICATION OF THE SIGN CONTROL ALGORITHM

4.1 Acceleration Noise as an Index of Performance

A quantitative measure of smoothness, comfort, and safety for a driver over an entire trip has been proposed by researchers^{18,22,30,33,47-49} in the traffic flow field. This measure is termed acceleration noise and is given by

$$\sigma^2 = \frac{1}{T} \int_0^T [a(t) - \bar{a}]^2 dt \quad (4-1)$$

where \bar{a} is the time average of $a(t)$ over the interval $(0, T)$.

From Eq.(4-1) it is seen that σ^2 is minimized when $a(t) = \text{constant} = \bar{a}$. This optimum cannot be practically attained since a driver is forced by the behavior of other drivers to decelerate and accelerate during his trip. Although it is the role of sign control to lessen these perturbations, they cannot be completely eliminated. Conceptually, the only way to completely avoid the effect of the car ahead is to allow sufficient intervehicle spacing so that any foreseeable deceleration disturbance would be cancelled by a succeeding acceleration before the gap is closed. Such a system is impractical and permits only very small values of flow.

The acceleration noise may also be expressed as

$$\sigma^2 = \frac{1}{T} \int_0^T [a(t)]^2 dt - [\bar{a}]^2 \quad (4-2)$$

Over a reasonably long trip the average acceleration given by

$$\bar{a} = \frac{1}{T} \int_0^T a(t) dt = \frac{v(T) - v(0)}{T}$$

will be small and the second term in Eq.(4-2) can be neglected giving

$$\sigma^2 = \frac{1}{T} \int_0^T [a(t)]^2 dt \quad (4-3)$$

For other reasons it is desirable to omit \bar{a} in the formulation of acceleration noise. It is unreasonable to consider two trips equally smooth if one has $a(t) \equiv 0$ and the other has $a(t) = \text{constant}$, yet both of these cases give $\sigma^2 = 0$ using Eq.(4-1). Henceforth the term acceleration noise will refer to that given by Eq.(4-3).

It is noted that the quantity T refers to the total duration of the trip and not only to the duration of non-zero acceleration. Thus σ^2 is more appropriately termed "average" acceleration noise. The average attribute allows the direct comparison of values of σ^2 for trips of different duration as is illustrated below. For the three acceleration waveforms depicted in Fig. 4-1, if all trips have the same duration $2T$, then the values of σ^2 are

$$\sigma_A^2 = \frac{1}{2} A^2$$

$$\sigma_B^2 = A^2$$

$$\sigma_C^2 = 2A^2$$

However if case A represents a trip of duration T then

$$\sigma_A^2 = A^2$$

and the shorter trip has the same value of σ^2 as case B.

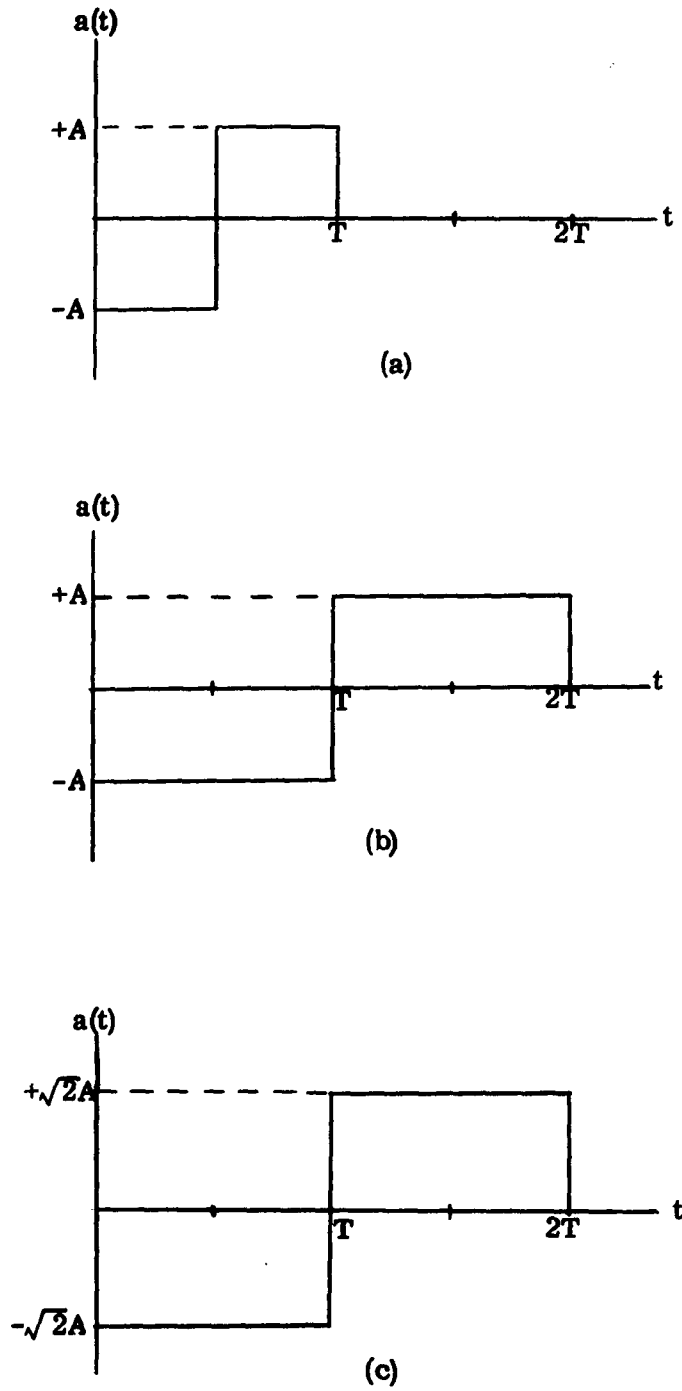


Fig. 4-1 Acceleration waveforms

In general a driver's trip, whether or not under sign control, can be separated into segments of time during which the vehicle is either accelerating or decelerating. The successive intervals of time alternate between acceleration and deceleration. It is assumed that the velocity changes over these successive intervals are independent in the sense that the deceleration undergone by a vehicle during one interval will not substantially affect its acceleration or deceleration in a succeeding interval. The intervals may be assumed to be separated by time periods of constant speed. Under these conditions it is meaningful to ask how the acceleration noise over each of the intervals contributes to the total. If T_i represents the end of the i -th interval then

$$\begin{aligned}
 \sigma^2 &= \frac{1}{T_n - T_0} \left[\int_{T_0}^{T_1} a^2(t) dt + \int_{T_1}^{T_2} a^2(t) dt + \dots + \int_{T_{n-1}}^{T_n} a^2(t) dt \right] \\
 &= \frac{1}{T_n - T_0} \left[(T_1 - T_0)\sigma_1^2 + (T_2 - T_1)\sigma_2^2 + \dots + (T_n - T_{n-1})\sigma_n^2 \right] \\
 &= \frac{1}{T} \sum_{i=1}^n (T_i - T_{i-1})\sigma_i^2 \tag{4-4}
 \end{aligned}$$

where σ_i^2 is the acceleration noise over the i -th interval.

From this analysis it is concluded that in order to minimize the acceleration noise over the entire trip it is sufficient to minimize the quantities

$$(T_i - T_{i-1})\sigma_i^2 = \int_{T_{i-1}}^{T_i} a^2(t) dt$$

over the i -th interval for each vehicle. This requires the assumption stated above that σ_i^2 and σ_j^2 can be minimized independently. This fact is utilized in the sign control algorithm.

4.2 Calculation of Optimum Deceleration for a Simple Highway Situation

The simple highway situation is considered in which all vehicles are initially traveling at the same constant speed, V_0 , although at different spacings. The leading vehicle then decelerates to a new steady-state speed V_f . In this case it is assumed that the following vehicles eventually reach an equilibrium speed of V_f at a smaller spacing appropriate to the new speed. The optimum manner in which these vehicles should make the transition from V_0 to V_f so that the integral $\int [a(t)]^2 dt$ is minimized for each vehicle is now considered.

Attention is directed to a particular following vehicle and the problem is formally stated as follows. Given two vehicles having initial speeds $v_1(0) = V_f$ and $v_2(0) = V_0 > V_f$ and initial spacing $x_1(0) - x_2(0) = S_0 > 0$ and given that $v_1(t) = V_f = \text{constant}$. Find the acceleration of the following vehicle, $a_2(t)$, such that $I = \int_0^T [a_2(t)]^2 dt$ is minimized subject to the final conditions $v_2(T) = V_f$ and $x_1(T) - x_2(T) = S_f < S_0$. It is noted that the upper limit is given as T rather than infinity since it is expected that the optimum $a_2(t)$ is a function of the upper limit. Also, the following vehicle may be any of the vehicles upstream of the leading vehicle, not necessarily vehicle 2 in the line.

To simplify the calculation the following normalized and equivalent problem is solved. Given a vehicle initially having $x(0) = 0$ and $v(0) = V_0$. Find the acceleration $a(t)$ to minimize $I = \int_0^T [a(t)]^2 dt$ and satisfy $x(T) = D$,

$v(T) = 0$. This problem is of the form

Minimize $I = \int_0^T f(t, x, \dot{x}, \ddot{x}) dt$ with
 respect to $x(t)$ subject to given values
 of x and \dot{x} at the upper and lower limits.

According to the calculus of variations⁵⁰ the twice-differentiable function $x(t)$ which extremizes I satisfies the differential equation

$$\frac{\partial f}{\partial x} - \frac{d}{dt} \left(\frac{\partial f}{\partial \dot{x}} \right) + \frac{d^2}{dt^2} \left(\frac{\partial f}{\partial \ddot{x}} \right) = 0$$

In the posed problem $f(t, x, \dot{x}, \ddot{x}) = [\ddot{x}(t)]^2$ and the above differential equation becomes

$$\frac{d^2}{dt^2} [2x(t)] = 0$$

which has the solution

$$\ddot{x}(t) = A + Bt \quad (4-5)$$

The final solution is found by solving for A and B in terms of the upper limit T , then evaluating I as a function of T , and then finding the value of T and hence the function $a(t)$ that extremizes I .

Integration of Eq.(4-5) and imposition of the initial values of x and v yields

$$v(t) = At + \frac{1}{2} Bt^2 + V_0 \quad (4-6a)$$

$$x(t) = \frac{1}{2} At^2 + 1/6 Bt^3 + V_0 t \quad (4-6b)$$

Substituting the required values of x and v at the upper limit gives the two equations

$$AT + \frac{1}{2} BT^2 + V_0 = 0$$

$$\frac{1}{2} AT^2 + 1/6 BT^3 + V_0 T = D$$

These two linear simultaneous equations in A and B yield the solutions

$$A = -\frac{2}{T^2} [2V_0T - 3D] \quad (4-7a)$$

$$B = \frac{6}{T^3} [V_0T - 2D] \quad (4-7b)$$

The integral to be extremized is

$$I = \int_0^T (A + Bt)^2 dt \quad (4-8)$$

When the values of A and B given by Eqs.(4-7) are substituted into Eq.(4-8)

and the integral is evaluated the result is

$$I = 4 \left\{ \frac{V_0^2}{T} - \frac{3DV_0}{T^2} + \frac{3D^2}{T^3} \right\} \quad (4-9)$$

$$\frac{dI}{dT} = -\frac{4V_0^2}{T^4} \left[T - \frac{3D}{V_0} \right]^2$$

Thus $dI/dT = 0$ at $T = 3D/V_0$ and at $T = \infty$. Also dI/dT is non-positive for all $T \geq 0$. Thus since $I \rightarrow +\infty$ for $T \rightarrow 0^+$ and $I \rightarrow 0^+$ for $T \rightarrow +\infty$ the shape of $I(T)$ is deduced as shown in Fig.4-2. The unconstrained minimum of I occurs at $T = +\infty$ where $I = 0$. However the expression for $v(t)$ given by Eq.(4-6a) becomes upon substitution of A and B from Eqs.(4-7)

$$v(t) = \frac{3}{T^3} (V_0T - 2D)(t - T)(t - t_0)$$

where

$$t_0 = \frac{V_0T^2}{3[V_0T - 2D]}$$

For $T > 3D/V_0$ it is found that t_0 is in the range $\frac{1}{3}T < t_0 < T$ giving a parabolic speed profile as shown in Fig.4-3. Since a negative speed is inadmissible

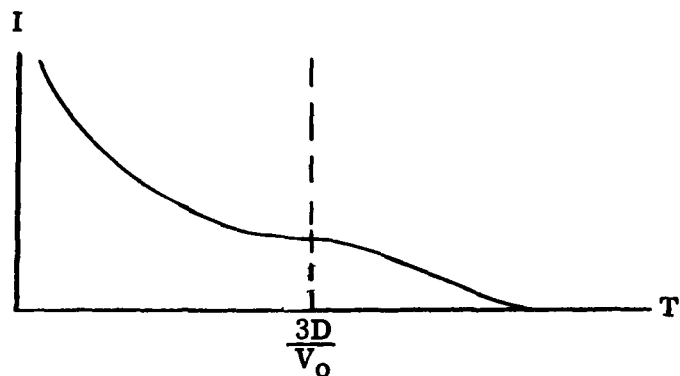


Fig.4-2 The integral $I(T)$

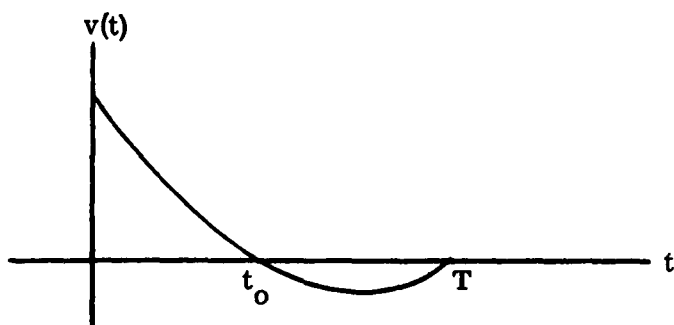


Fig.4-3 Speed profile for $\frac{1}{3} T < t_0 < T$

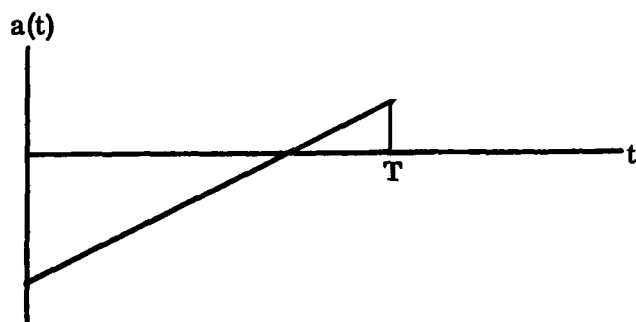


Fig.4-4 Acceleration waveform for $T > 3D/V_0$

the unconstrained solution cannot be accepted. For an acceleration of the form $A+Bt$ the final time must be limited to $T \leq 3D/V_0$. This may also be seen by examining the expression for the acceleration at the final time

$$a(T) = A + BT = \frac{2}{T^2}(V_0T - 3D)$$

For $T > 3D/V_0$ it is seen that $a(T) > 0$ implying an acceleration waveform as shown in Fig. 4-4. Since $v(T) = 0$ the $a(t)$ of Fig. 4-4 would require that $v(t) < 0$ for some $t < T$.

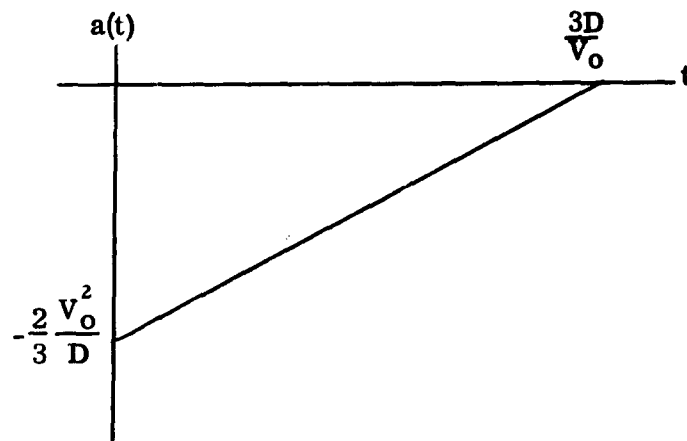
Since $I(T)$ is monotonic decreasing, as previously shown, the optimum acceleration waveform is obtained by choosing $T = 3D/V_0$. This yields the motion given by

$$\begin{aligned} a(t) &= \frac{2}{9} \frac{V_0^3}{D^2} \left[t - \frac{3D}{V_0} \right] \\ v(t) &= \frac{1}{9} \frac{V_0^3}{D^2} \left[t - \frac{3D}{V_0} \right]^2 \\ x(t) &= \frac{1}{27} \frac{V_0^3}{D^2} \left[t - \frac{3D}{V_0} \right]^3 + D \end{aligned} \quad (4-10)$$

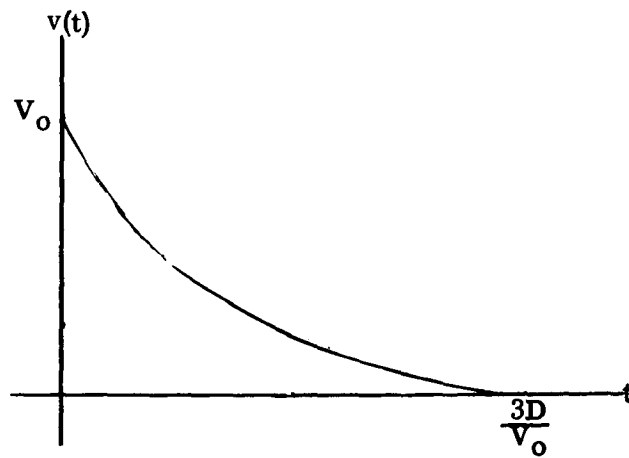
for $0 \leq t \leq 3D/V_0$ as shown in Fig. 4-5. Upon substitution of $T = 3D/V_0$ into Eq.(4-9) the integral I becomes

$$I = \frac{4}{9} \frac{V_0^3}{D}$$

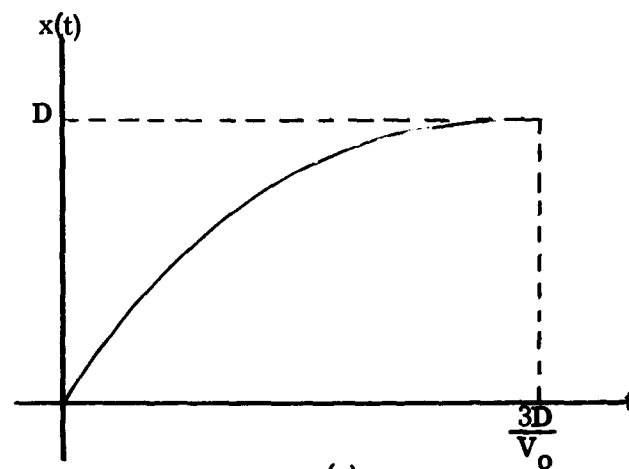
It is noted that the minimizing function for the motion constrained by $v(t) \geq 0$ for $(0, T)$ has not been obtained. Rather the minimizing function for the unconstrained problem has been found and the result has been modified to



(a)



(b)



(c)

Fig. 4-5 Optimal motion: (a) acceleration, (b) velocity, (c) position

assure $v(t) \geq 0$ for $(0, T)$. The inclusion of such a constraint in the problem would make it impossible to find the solution by other than numerically based methods. However the result does yield a smaller value of I than any other function that is restricted to the interval $(0, 3D/V_0)$. Furthermore, as will be explained, there are reasons for believing that the solution obtained is also the solution to the constrained problem.

When the acceleration is assumed to be constant in $(0, T)$ then the solution satisfying the initial and final conditions on spacing and speed is $a(t) = -V_0^2/2D$ for $0 \leq t \leq 2D/V_0$. For the constant acceleration case it is found that $I = V_0^3/2D$. When the acceleration is assumed piecewise constant over two equal intervals of time then the solution that minimizes I and has $v(t) \geq 0$ for all t can be shown to yield $I = \frac{25}{54} V_0^3/D$. In addition an exponential acceleration, with time-constant D/V_0 requiring infinite time, yields $I = V_0^3/2D$. These solutions are sketched in Fig. 4-6 and summarized in Table 4-1. For comparison the ramp acceleration and the piecewise constant acceleration are shown on the same graph.

The fact that the piecewise constant acceleration curve closely follows the ramp acceleration curve supports the contention that the ramp acceleration is the minimizing function. It is expected that if a piecewise constant acceleration having three intervals were assumed, the resulting waveform that minimizes the integral I would more closely follow the ramp.

For the first problem posed of a vehicle decelerating from V_0 to V_f while its spacing to a downstream car changes from S_0 to S_f , the solution

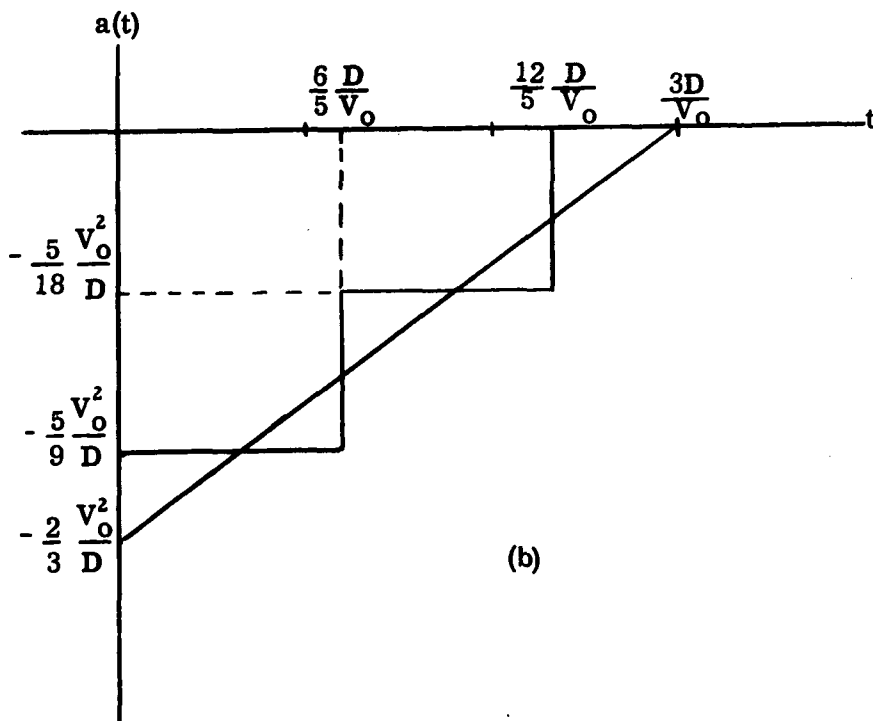
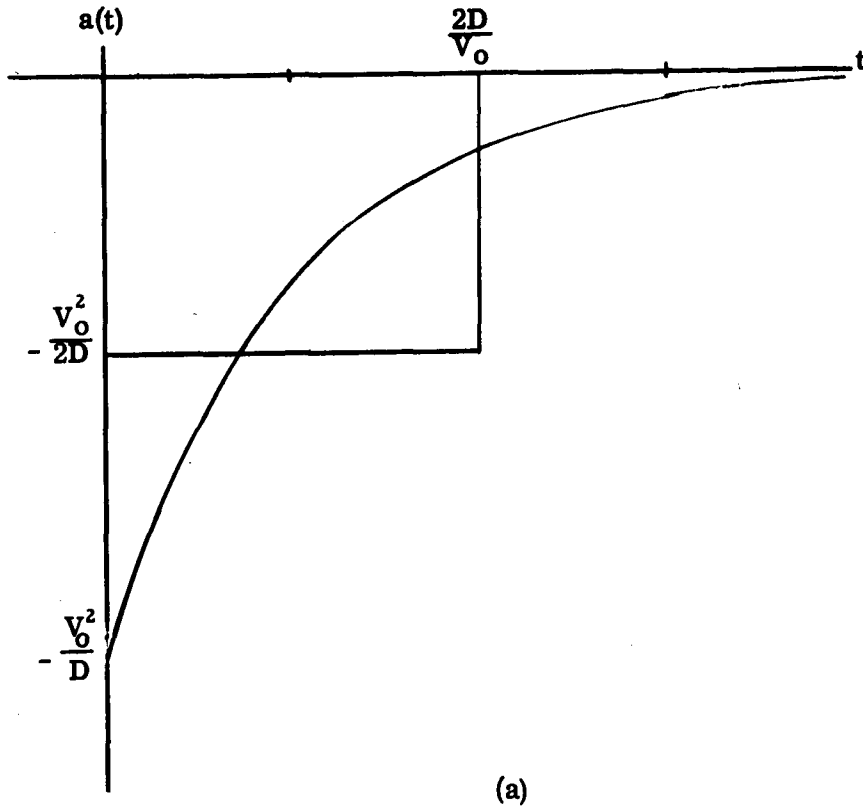


Fig. 4-6 Acceleration waveforms for optimal and sub-optimal solutions

Table 4. 1 - Value of the Integral I for Optimal and Sub-optimal Waveforms

Acceleration Waveform	Time Duration	I	I/I _{ramp}
exponential	∞	$\frac{1}{2} V_0^3/D$	1.125
constant	$2D/V_0$	$\frac{1}{2} V_0^3/D$	1.125
piecewise constant	$\frac{12}{5} D/V_0$	$\frac{25}{54} V_0^3/D$	1.042
ramp	$3D/V_0$	$\frac{4}{9} V_0^3/D$	1.0

analogous to Eqs.(4-10) is found to be

$$a(t) = -\frac{2}{9} \frac{(\Delta V)^3}{(\Delta S)^2} \left[t - \frac{3\Delta S}{\Delta V} \right]$$

$$v(t) = -\frac{1}{9} \frac{(\Delta V)^3}{(\Delta S)^2} \left[t - \frac{3\Delta S}{\Delta V} \right]^2 + V_f$$

$$x(t) = -\frac{1}{27} \frac{(\Delta V)^3}{(\Delta S)^2} \left[t - \frac{3\Delta S}{\Delta V} \right]^3 + V_f t - \Delta S$$

for $0 \leq t \leq 3\Delta S/\Delta V$ and where $\Delta V = V_f - V_o$ and $\Delta S = S_f - S_o$. For this motion the integral I is

$$I = \frac{4}{9} \frac{(\Delta V)^3}{\Delta S}$$

For the constant deceleration case the problem is posed as follows.

Given two vehicles (not necessarily consecutive) having initial speeds

$v_1(0) = V_f$ and $v_2(0) = V_o$ and initial spacing $x_1(0) - x_2(0) = S_o > 0$ and given

that $v_1(t) = V_f = \text{constant}$. Find the constant acceleration, A, of the second vehicle in the interval $(0, T)$ that satisfies $v_2(T) = V_f$ and $x_1(T) - x_2(T) = S_f$.

To achieve the desired speed change, $V_f - V_o$, it is required that $A = (V_f - V_o)/T$. Since $a_2(t) = A = \text{constant}$, the speed $v_2(t)$ is a ramp function as shown in Fig. 4-7. The negative of the shaded area equals the change in spacing. Thus $S_f - S_o = \frac{1}{2}T(V_f - V_o)$ and hence

$$T = 2(S_f - S_o)/(V_f - V_o).$$

Substituting this value of T in the expression for A gives the result

$$A = \frac{1}{2}(\Delta V)^2/(\Delta S) \quad \text{for } 0 \leq t \leq T$$

$$T = 2(\Delta S)/(\Delta V)$$

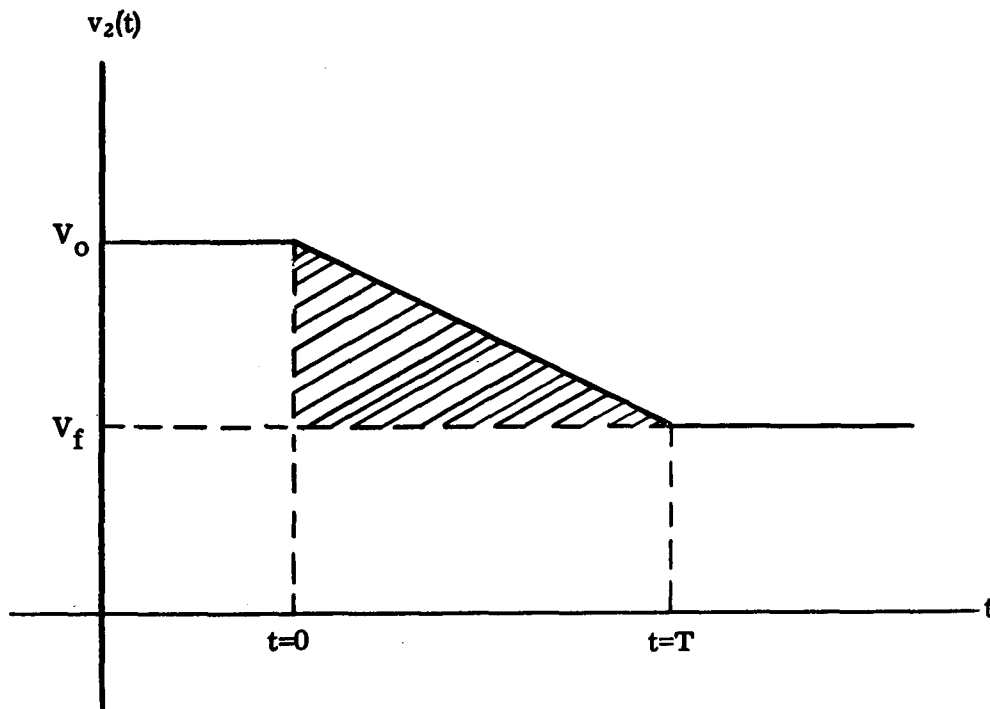


Fig. 4-7 Speed of following vehicle for constant deceleration case

where $\Delta V = V_f - V_0$ and $\Delta S = S_f - S_0$. The integral I for this case becomes

$$I = \frac{1}{2}(\Delta V)^3 / (\Delta S)$$

It is noted that for the ramp acceleration there are an infinite number of solutions that satisfy the initial and final conditions while for the constant acceleration the solution is unique.

As shown in Table 4-1 the waveforms considered yield values of I not substantially different from the minimum value obtained for the ramp function. For this reason and for reasons of mathematical tractability the constant acceleration waveform is chosen for the sign control application. The same methods to be used could also be employed with the ramp function with the only difference being a substantial increase in program complexity.

4.3 The Sign Control Algorithm as Applied to a Simple Highway Situation

Assumptions It is assumed that the simple highway situation exists in which a line of vehicles are initially all traveling at speed V_0 although at different spacings. The sequence of detectors spanned by these vehicles all register the speed V_0 . It is assumed that the lead vehicle then decelerates to V_f and some time later passes a detector where the decrease in speed is registered.

At the next analysis time the settings for the advisory signs are computed on the basis of existing traffic conditions. These settings are valid for only 5 seconds after which time conditions are re-evaluated to determine new sign settings.

The basis for determining the sign settings is the temporary assumption that all vehicles upstream of the lead vehicle will eventually be forced to decelerate to V_f . The desired deceleration for these vehicles has previously been determined to be a constant given by $A_d = \frac{1}{2}(\Delta V)^2/(\Delta S)$ over an interval of time $T = 2(\Delta S)/(\Delta V)$. The approach used is to determine the sign settings such that the average deceleration due to the sign equals the desired deceleration for vehicles traversing an entire section under sign control.

Therefore to determine the advisory speed setting for the i -th sign, attention is focused on an imaginary vehicle located at the $(i-1)$ -th detector that is 0.1 mile upstream of the i -th sign. It is this vehicle that responds to

the initial setting of the i -th sign during the first 5 secs. of its journey on the i -th section.

The desired deceleration of the imaginary vehicle, A_d , will be expressed in terms of the measured section speeds and densities. This car's predicted average deceleration, A_a , will be derived from an assumed sign-following law as a function of its speed when entering the section and the speed setting of the advisory sign located at the next section boundary. Equating A_d to A_a will yield the solution for the advisory sign setting in terms of the detector outputs.

Computation of Desired Deceleration A section of highway is depicted in Fig. 4-8. The vertical delineators indicate the detector locations, spaced by 0.1 mile. The k -th detector stores VD_k , the velocity of the vehicle most recently passing the detector, and ND_k , the number of vehicles currently on the k -th section of roadway. The advisory signs are also spaced by 0.1 mile and the speed setting is represented as V_{s_i} for the i -th sign.

In accordance with the simple highway situation previously described it is assumed that at a particular sampling time $VD_j = V_f$ and all other upstream detectors register $VD_k = V_o$, where $V_f < V_o$.

To determine the speed setting for the i -th sign it is necessary that the desired deceleration of an imaginary vehicle located 0.1 mile upstream of sign i be expressed in terms of the detector outputs. The desired deceleration for this vehicle is

$$A_d = \frac{1}{2}(\Delta V)^2 / (\Delta S) \quad (4-11)$$

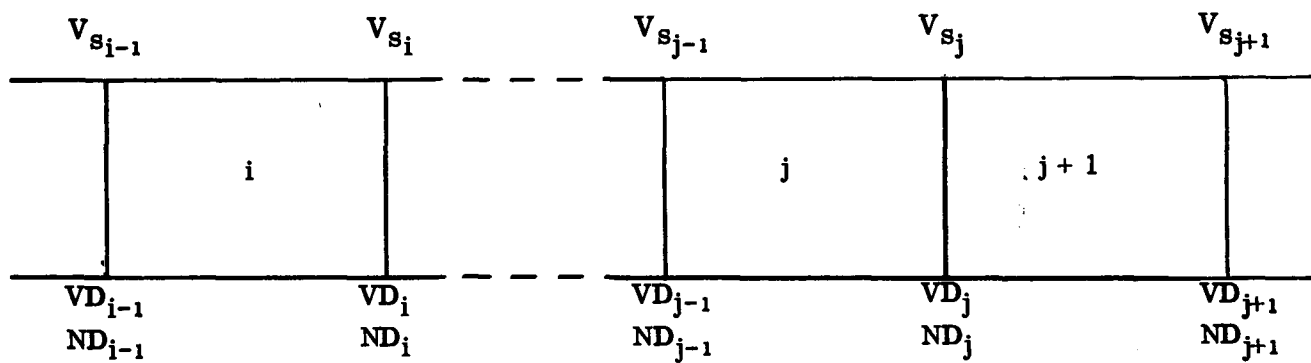


Fig. 4-8 Representation of highway and location of detectors

for an interval of time $T = 2(\Delta S)/(\Delta V)$ where $\Delta V = V_f - V_o$ is the difference between the final speed after a slowdown and the initial speed and $\Delta S = S_f - S_o$ is the difference between final and initial spacings from the vehicle initiating the slowdown.

In this example the initial speed is $V_o = VD_{i-1}$ and the final speed is $V_f = VD_j$ so that

$$\Delta V = V_f - V_o = VD_j - VD_{i-1} \quad (4-12)$$

It is assumed that the vehicle that initiated the slowdown has just passed detector j . Thus the initial spacing equals the distance between detector $i-1$ and detector j , that is

$$S_o = 528 (j - i + 1) \quad \text{feet} \quad (4-13)$$

The final spacing between the imaginary vehicle currently at detector $i-1$ and the vehicle assumed to be initiating the slowdown is determined from the number of vehicles between them and by the final speed. At the conclusion of the slowdown it is desired that each vehicle be at the spacing appropriate to its new speed. For the i -th vehicle this desired spacing is

$$\begin{aligned} DD_i &= HDRV_i \times V_f + 20 \\ &= HDRV_i \times VD_j + 20 \quad \text{feet} \end{aligned}$$

where $HDRV_i$ is the headway factor generated for the i -th vehicle.

The ND_k vehicles in section k initially occupy 528 feet. After the slowdown they will occupy less space. For these vehicles the sum of their desired final spacings to the front bumper of the car ahead is expressed as

$$S_f(k) = \sum_{i=1}^{ND_k} (HDRV_i \times VD_j + 20) = 20 ND_k + VD_j \sum_{i=1}^{ND_k} HDRV_i \quad (4-14)$$

where the sum is over the ND_k vehicles on the section. Since these vehicles initially occupied a 528 foot section

$$528 = \sum_{i=1}^{ND_k} (HDRV_i \times v_i + 20) = 20 ND_k + \sum_{i=1}^{ND_k} HDRV_i \times v_i \quad (4-15)$$

where v_i is the initial speed of the i -th vehicle. The v_i are approximated by the estimated average section speed, V_k , determined from the detectors at the section boundaries to be

$$V_k = (VD_{k-1} + VD_k)/2 \quad (4-16)$$

Replacing v_i in Eq.(4-15) by V_k from Eq.(4-16) gives

$$528 = 20 ND_k + V_k \sum_{i=1}^{ND_k} HDRV_i \quad (4-17)$$

From Eq.(4-17)

$$\sum_{i=1}^{ND_k} HDRV_i = \frac{528 - 20 ND_k}{V_k} \quad (4-18)$$

Substituting from Eq.(4-18) into Eq.(4-14) gives

$$S_f(k) = 20 ND_k + \frac{VD_j}{V_k} (528 - 20 ND_k) \quad (4-19)$$

or, using Eq.(4-16),

$$S_f(k) = 20 ND_k + \frac{2 VD_j}{VD_{k-1} + VD_k} (528 - 20 ND_k) \quad (4-20)$$

Thus the desired cumulative final spacing for the vehicles initially on

sections i through j is

$$S_f = \sum_{k=1}^j S_f(k) \quad (4-21)$$

Substitution of the derived quantities into Eq.(4-21) gives, from Eq.(4-11),

$$A_d = \frac{1}{2} \frac{(VD_j - VD_{i-1})^2}{\sum_{k=1}^j \left[20 ND_k + \frac{2 VD_j}{VD_{k-1} + VD_k} (528 - 20 ND_k) \right] - 528(j-i+1)} \quad (4-22)$$

This is the desired acceleration for a vehicle traversing the i -th section in response to a slowdown detected at the j -th detector.

Predicted Average Deceleration from Sign-following Model The

average deceleration of a vehicle over a section of highway in response to a sign is determined from the sign-following model employed to represent the dynamic response. Knowledge of car-following theory leads to a realistic sign-following model in the form of a stimulus-response equation. The sign-following model chosen is given by

$$\dot{v}(t) = \beta[V_s - v(t - \tau_s)] \quad (4-23)$$

The response is the acceleration at the current time and the stimulus is the difference between V_s , the speed indication of the sign that the driver sees at time $t - \tau_s$ and $v(t - \tau_s)$, the vehicle's speed at $t - \tau_s$.

To facilitate further analysis of the model in order to obtain a sign setting algorithm the model is modified to become

$$\dot{v}(t) = \begin{cases} \beta[V_s - v(t - \tau_s)] & \text{for } t \geq \tau_s \\ 0 & \text{for } t < \tau_s \end{cases} \quad (4-24)$$

This modification has little effect on the dynamics of the response since in most cases the quantity $\dot{v}(t)$ is small for $0 \leq t \leq \tau_s$ because near the end of the previous section of highway the vehicle's speed will be nearly equal to the setting of the previous sign.

Values of β and τ_s are chosen to be equal to the corresponding parameters in the analogous car-following law for a very conservative driver. From data in Table I of Helly's paper²³ a reasonable choice is $\beta = 0.2 \text{ sec}^{-1}$, $\tau_s = 2.0 \text{ sec}$.

The average deceleration of a vehicle traversing section i under control of the i -th sign may be found from Eq.(4-24) if the initial condition on the vehicle's speed, $v(0)$, and the speed setting of the sign, V_s , are known.

However for a given initial speed it is desired to find the value of V_s that yields a specified average acceleration. By an iterative procedure the value of V_s can be found that yields the specified average acceleration. Although this procedure may be readily implemented on a digital computer, a more direct solution for V_s would save substantial computation time. To this end an approximate solution is developed.

Let $v(0) = V_0$ and introduce the normalization

$$r(t) = \frac{v(t) - V_0}{V_s - V_0} \quad (4-25)$$

into Eq.(4-24) yields the differential equation

$$\dot{r}(t) = \beta[1 - r(t-\tau)] \quad (4-26)$$

where τ stands for τ_s and

$$r(0) = 0$$

$$r(\infty) = 1$$

The solution of Eq.(4-26) is of the form

$$r(t) = f(\beta, \tau, t)$$

independent of V_0 and V_s with $f(\beta, \tau, 0) = 0$ and $f(\beta, \tau, \infty) = 1$. Thus,

from Eq.(4-25),

$$v(t) = (V_s - V_0) f(\beta, \tau, t) + V_0 \quad (4-27)$$

as is verified by the solution of Eq.(4-24) found by Laplace transform techniques to be

$$v(t) = \begin{cases} V_0 & \text{for } 0 \leq t \leq \tau_s \\ V_0 + (V_0 - V_s) \sum_{k=1}^{[t/\tau_s]} (-\beta)^k \frac{1}{k!} (t - k\tau_s)^k & \text{for } t > \tau_s \end{cases} \quad (4-28)$$

where $[t/\tau_s]$ is the largest integer in t/τ_s .

The average acceleration of a vehicle on a 528 foot section of highway is given by

$$A_a = \frac{v(T) - V_0}{T} \quad (4-29)$$

where $v(0) = V_0$ and T is the section travel time given by the solution of

$$x(T) = \int_0^T v(t) dt = 528$$

From Eq.(4-27)

$$v(T) - V_0 = (V_s - V_0) f(\beta, \tau, T) \quad (4-30)$$

Typically the average speed on a section under sign control is in the range $40 < \bar{v} < 55$ mi/hr. In this range it is found that $f(\beta, \tau, T)$ varies by approximately 10 percent about a mean value. For an average speed of 55 mi/hr $T = 6.54$ sec. and $f(\beta, \tau, T) = 0.78$ while for an average speed of 40 mi/hr $T = 9.0$ sec. and $f(\beta, \tau, T) = 0.935$.

For the purpose of developing an approximation for average acceleration it is reasonable to approximate Eq.(4-30) by

$$v(T) - V_0 \approx C(V_S - V_0) \quad (4-31)$$

From Eq.(4-27) an expression for the average velocity on a section is

$$\bar{v} = \frac{1}{T} \int_0^T v(t) dt = V_0 + \left[\frac{1}{T} \int_0^T f(\beta, \tau, t) dt \right] (V_S - V_0)$$

The value of $\frac{1}{T} \int_0^T f(\beta, \tau, t) dt$ varies by about 20 percent about a mean value for the typical range of T . Nevertheless a value of K is chosen that best approximates the average value of f over the typical range of T to give

$$\bar{v} \approx V_0 + K(V_S - V_0)$$

Thus, the travel time on a 528 foot section is approximated by

$$T = \frac{528}{\bar{v}} \approx \frac{528}{V_0 + K(V_S - V_0)} \quad (4-32)$$

Substituting the results of Eq.(4-31) and Eq.(4-32) into Eq.(4-29) gives

$$A_a \approx C_1(V_S - V_0)^2 + C_2 V_0(V_S - V_0) \quad (4-33)$$

as a reasonable form for the approximating function. Since $A_a = A_d$ in a

sign control application both A_a and V_o are known. Once C_1 and C_2 are determined the quadratic equation (4-33) may easily be solved for V_s .

To determine C_1 and C_2 several hundred pairs of V_o and V_s were chosen in the range $88 \geq V_s + 40 \geq V_o \geq V_s + 4 \geq 34$. For each pair (V_o^i, V_s^i) the exact solution for average acceleration, A_T^i , was computed by the method previously outlined. The values of C_1 and C_2 were determined that minimize the squared error

$$\epsilon^2 = \sum_{i=1}^N (A_T^i - A_a^i)^2$$

where A_a^i is the approximation given by Eq.(4-33) for the i -th pair (V_o^i, V_s^i) .

The resulting approximation is

$$A_a \approx 0.000452 (V_o - V_s)^2 + 0.001581 V_o (V_s - V_o) \quad (4-34)$$

with an average absolute error of 5.23 percent.

By trying other approximating functions it is found that the choice

$$A_a = -0.00087(V_o^2 - V_s^2) \quad (4-35)$$

gives a slightly larger squared error than Eq.(4-34) and a slightly smaller average absolute error of 4.95 percent. Since Eq.(4-35) is algebraically simpler than Eq.(4-34), it is chosen for the sign control application.

When applying Eq.(4-35) to the setting of the i -th sign, $V_o = VD_{i-1}$ and $V_s = V_{s_i}$ yielding

$$A_a = -0.00087 (VD_{i-1}^2 - V_{s_i}^2) \quad (4-36)$$

for the predicted average acceleration of a vehicle traversing section i under control of the i -th sign.

Solution for Sign Setting

Expressions for the desired average

acceleration, A_d , and the predicted average deceleration, A_a , have been obtained. The value of the setting for the i -th sign, V_{s_i} , is found by equating A_d from Eq.(4-22) to A_a from Eq.(4-36) with $V_s = V_{s_i}$. The solution for V_{s_i} is

$$V_{s_i} = \left\{ VD_{i-1}^2 + \frac{575(VD_j - VD_{i-1})^2}{\sum_{k=i}^j \left[20 ND_k + \frac{2VD_j(528-20ND_k)}{VD_{k-1} + VD_k} \right] - 528(j-i+1)} \right\}^{\frac{1}{2}} \quad (4-37)$$

where VD_j is the speed indication of the downstream detector that is temporarily assumed to equal the final velocity of the upstream vehicles after the slowdown.

Rounding of Sign Settings

For practical reasons the sign-

settings are rounded to the standard 5 mi/hr increments. In addition, a sign is not turned on if only a minor response to the calculated setting is anticipated.

After computing the exact sign settings via Eq.(4-37) each sign is examined and those that are off are skipped. If the i -th sign is on, it is then ascertained whether a significant response is expected to the sign setting. If $VD_{i-1} - V_{s_i} \leq 2.5$ mi/hr the i -th sign is turned off. If $VD_{i-1} - V_{s_i} > 2.5$ mi/hr the sign setting, V_{s_i} , is rounded to the nearest 5 mi/hr. If, for the rounded sign setting, the quantity $VD_{i-1} - V_{s_i} \leq 2.5$ mi/hr the sign is turned off. If $VD_{i-1} - V_{s_i} > 2.5$ mi/hr, for the rounded value, then the rounded value is retained as the current sign setting.

Example of Sign Settings for a Simple Highway Situation An

example from a simulation run with 80 vehicles is presented. All vehicles enter the roadway at a speed of 50 mi/hr and continue at this constant speed until $t = 120$ sec. At this time the first vehicle decelerates to 35 mi/hr at a rate of -3.0 ft/sec^2 .

At $t = 125$ sec. the slowdown is detected and one sign is set; at $t = 130$ sec. two signs are set; and at $t = 135$ sec. five signs are set. The situation at $t = 135$ sec. is examined in more detail.

The detector information available at $t = 135$ sec. is shown tabulated below. The value of $VD(I)$ is rounded to the nearest integer.

I	ND (I) (vehicles)	VD (I) (mi/hr)
11	4	50
12	5	50
13	5	50
14	5	50
15	3	48
16	4	46
17	4	35
18	6	34
19	2	--

From several computer runs it is found that the acceleration noise is minimum when the constant 575.0 in Eq.(4-37) is replaced by a constant

between 600.0 and 625.0. For the example under consideration this constant is taken to be 625.0. Consequently the appropriate equation for determining the setting of the i -th sign due to a slowdown detected at VD_j

is

$$V_{S_i} = \left\{ VD_{i-1}^2 + \frac{625.0(VD_j - VD_{i-1})^2}{\sum_{k=1}^j \left[20ND_k + \frac{2VD_j(528-20ND_k)}{VD_{k-1} + VD_k} \right] - 528(j-i+1)} \right\}^{\frac{1}{2}} \quad (4-38)$$

The data from several simulations indicate that it is preferable not to set the sign that is located directly at the point where the slowdown is detected. It is found that the setting of this sign often retards vehicles in recovering from a slowdown after the vehicle causing the slowdown increases its speed. Therefore in this example sign 18 is skipped.

In determining the exact setting (before rounding) for sign 17 the appropriate values to substitute into Eq.(4-38) are

$$j = 18$$

$$i = 17$$

$$VD_{18} = 66.9 \text{ ft/sec (46 mi/hr)}$$

$$VD_{17} = 50.9 \text{ ft/sec (35 mi/hr)}$$

$$VD_{18} = 50.4 \text{ ft/sec (34 mi/hr)}$$

$$ND_{17} = 4$$

$$ND_{18} = 6$$

Substituting these values into Eq.(4-38) gives $V_{S_{17}} = 44.0 \text{ ft/sec}$ for the setting of sign 17.

To determine the exact setting for sign 14, for example, the values $j=18$ and $i=14$ should be used in Eq.(4-38). The value $j=18$ should be used in finding V_{S_i} for all values of i less than j .

In the manner described above the exact settings of the other upstream signs are determined as shown tabulated below. The results of the rounding algorithm are also tabulated.

Sign Number	Speed Setting (ft/sec) (mi/hr)		Rounded Setting (mi/hr)
17	44.0	30.0	30
16	60.0	40.9	40
15	66.1	45.1	45
14	68.3	46.5	45
13	69.4	47.3	45
12	70.2	47.9	OFF

It is noted that sign 12 is turned off because $VD_{11} - V_{S_{12}} = 50.0 - 47.9 = 2.1$ mi/hr is less than 2.5 mi/hr. That is, the sign is not turned on because the vehicle 528 ft upstream of the sign is traveling at a speed only slightly greater than the computed sign setting.

4.4 Application of Sign Control to a General Highway Situation

Introduction In the simplified highway situation previously described, the speed indications of the detectors are monotone decreasing in the downstream direction. This is intended to represent only an isolated component of the traffic pattern on an actual highway.

Sampling of the detector speed outputs at a random time generally yields a speed profile exhibiting peaks and valleys. A typical case is shown in Fig. 4-9.

General Procedure for Sign Setting The heuristic procedure for determining the sign settings is described for a typical case for which $VD(I)$ are as shown in Fig. 4-9. The detector speed outputs $VD(I)$ are scanned, starting at $VD(7)$, to locate the first "local minimum" (LM). A local minimum occurs at detector K if both

$$VD(K-1) > VD(K)$$

$$VD(K+1) \geq VD(K)$$

at satisfied. The three local minima are identified by arrows on Fig. 4-9.

The first LM in Fig. 4-9 is $VD(10)$. Sign 10 is not set since it is located at the local minimum. Sign settings for signs 9 and 8 are determined from Eq.(4-38) using $j = 10$ and the appropriate values for VD and ND . That is, V_{s_9} and V_{s_8} are computed on the basis of the LM of $VD(10)$. Sign 9 is set to 65.9 ft/sec and sign 8 is set to 68.7 ft/sec.

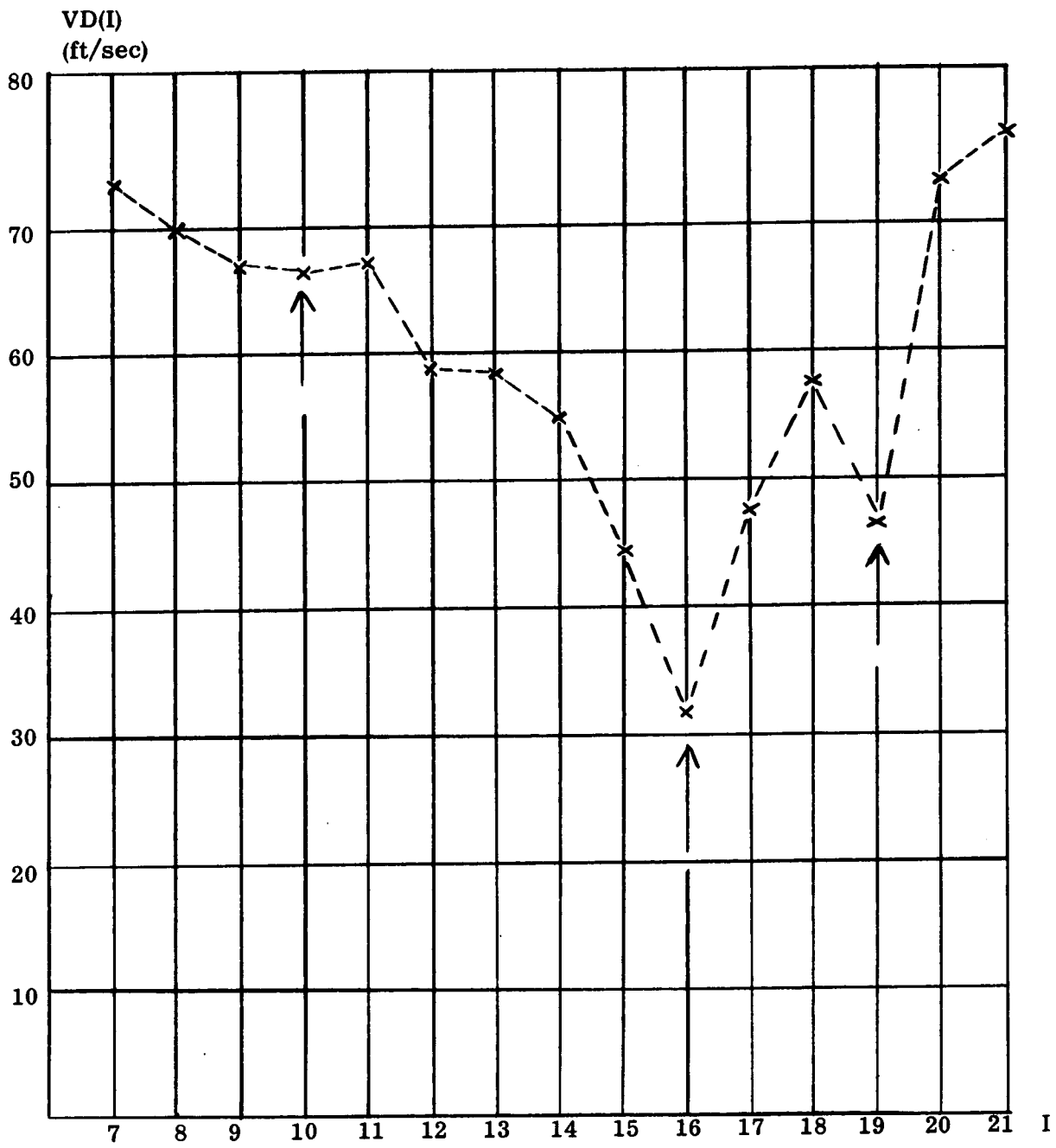


Fig. 4-9 Detector Speed Outputs for a Typical Case

The scan is continued and the second LM is found to be VD(16). Sign 16 is not set due to this local minimum because the LM occurs at the location of this sign. Signs 15 back through 8 are now considered on the basis of a slowdown assumed to originate from the second LM of VD(16). Thus, using the value $j=16$ in Eq.(4-38), the computed settings (ft/sec) for signs 15 back through 10 are

$$V_{S_{15}} = 38.5$$

$$V_{S_{14}} = 48.2$$

$$V_{S_{13}} = 51.9$$

$$V_{S_{12}} = 59.6$$

$$V_{S_{11}} = 60.8$$

$$V_{S_{10}} = 62.2$$

Sign 9 was considered previously due to the first LM of VD(10). However, since the second LM is of smaller value, a tentative setting is computed using $j=16$. This computation yields 65.4 ft/sec. Since 65.4 is less than the previously computed setting of 65.9 it is concluded that the second LM has the stronger influence on vehicles on section 10. Hence sign 9 is reset to $V_{S_9} = 65.4$.

Similarly a tentative setting for sign 8 is computed, using $j=16$, and the result is 69.0. Sign 8 is not reset because the previously determined setting of 68.7 ft/sec is less than 69.0.

The third LM is found at VD(19). Sign 19 is not considered and the setting for sign 18 is found to be $V_{S_{18}} = 47.6$ using $j=19$ in Eq.(4-38).

Sign 17 is not considered because $VD(16)$ is less than the speed at the third LM of $VD(19)$. Thus, cars on section 17 will not be forced to decelerate due to the LM of $VD(19)$. For the same reason sign 16 is not considered.

Sign 15 has been previously considered due to a LM further upstream of smaller value, i.e. $VD(16)$. For this reason the setting of sign 15 is considered to be valid. Furthermore, additional scanning of signs further upstream due to this LM is suppressed and the search for the next LM is initiated.

CHAPTER 5

SIMULATION RESULTS AND DISCUSSION

5.1 Prefatory Remarks

This chapter concludes the study of the proposed advisory speed sign system by (1) presenting the numerical results from the computer simulation program, (2) analyzing and interpreting these results, and (3) assessing the potential success of the system in improving the level of service afforded to drivers on an urban expressway under heavy traffic conditions.

The primary indicator of the degree of comfort and safety for a driver during his trip is the acceleration noise (see Section 4.1). The sign control algorithm has been designed to minimize this quantitative performance index. Another indicator of the improvement in the quality of traffic flow due to sign control is the total stopped time. For the main part, the utility of the system will be judged by the extent to which sign control reduces both the acceleration noise and the total stopped time.

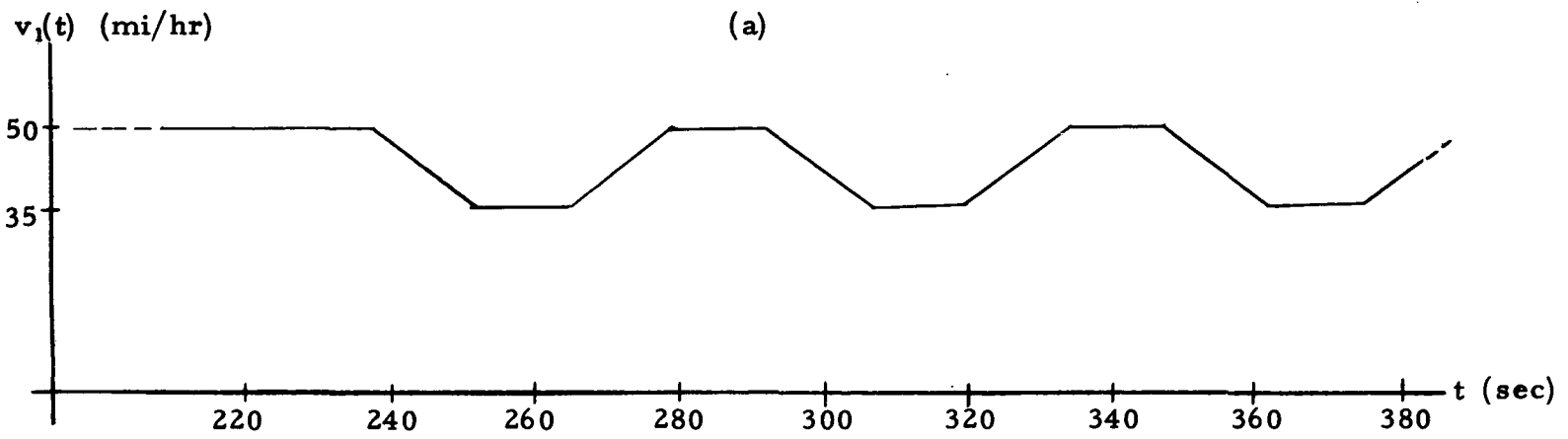
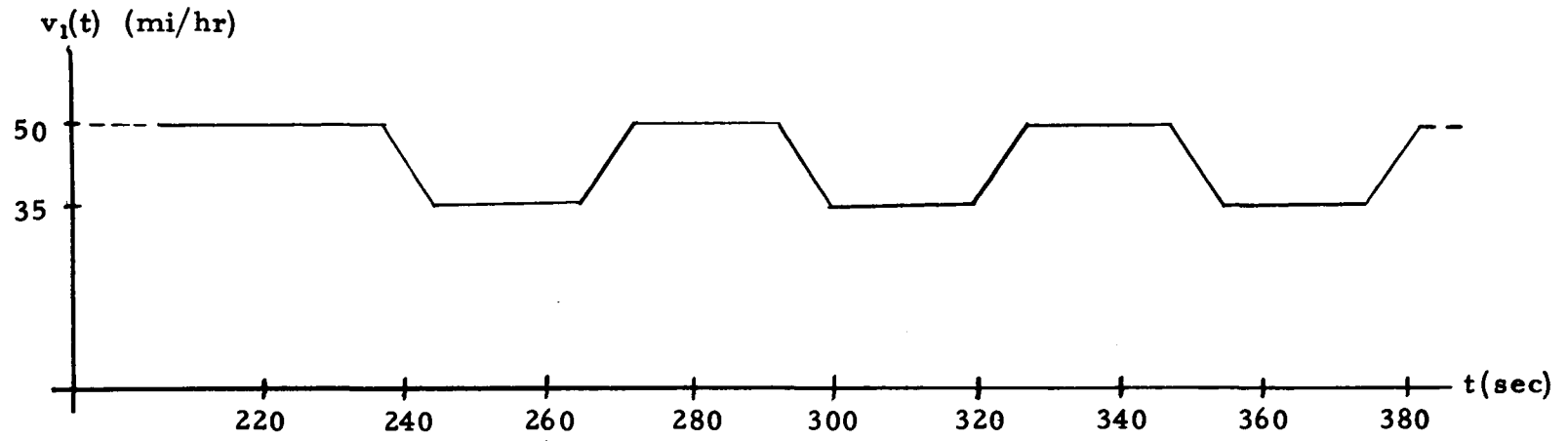
In each of the simulation runs 80 vehicles traverse an eight mile length of roadway. The 80 vehicles occupy approximately two miles of the roadway at any time.

One distinguishing characteristic of a sample population of 80 vehicles is the average headway factor associated with these drivers (see Section 3.2). The average headway factor is determined from the specified demand, D , which is an input parameter in the simulation. At a specified entering flow rate there is no typical sample of 80 vehicles from which a reliable estimate of the improvements due to sign control can be obtained. It is therefore necessary to generate several driver samples at each flow rate and to assess both the performance of the system with the individual samples and the average performance over all the samples.

The effects of sign control on acceleration noise and stopped time are obtained by comparing the results of a simulation run without sign control to those with sign control for the same driver sample. Under sign control the compliance level, which determines the fraction of drivers that heed the sign settings, is varied from 1.0 to 0.2 and the effect of non-perfect compliance is noted. The effect of the flow rate and the compliance level on the duty-cycle of the advisory signs is shown.

5.2 Maneuvers of the lead vehicle

The lead vehicle begins its maneuver shortly after all vehicles have entered the highway. The simulation has been run for two different repetitive maneuvers differing primarily in the magnitudes of the programmed decelerations and accelerations. In both maneuvers the lead vehicle undergoes a constant deceleration, from 50 mi/hr to 35 mi/hr, followed by an interval of constant speed, then a constant acceleration back to 50 mi/hr followed by an interval of constant speed. This motion is repeated with a period of 55 seconds until the vehicle reaches the end of the highway runoff section. The accelerations used in maneuver 1 are $\pm 3.0 \text{ ft/sec}^2$ whereas maneuver 2 uses $\pm 1.5 \text{ ft/sec}^2$. These maneuvers, which initiate the responses of the following vehicles, are shown in Fig.5-1.



(b)

Fig. 5-1 Lead vehicle motion (a) maneuver 1, (b) maneuver 2

5.3 Numerical Results

The results of the individual simulation runs without sign control and those with sign control at 100 percent compliance are shown in Tables 5-1 through 5-8. Each table corresponds to a particular value of demand, D .

Table 5-1, for $D = 2300$, shows a dramatic reduction in stopped time and acceleration noise due to sign control. The improvement is consistently high* for both lead car maneuvers and for each of the three driver samples. Maneuver 1, being more severe than maneuver 2, yields larger values of stopped time and acceleration noise both with and without sign control. The positive correlation between stopped time and acceleration noise is noted for the different driver samples. Thus, stopped time is also a correlate of the "smoothness" of flow, at least for those high values of D where the stopped time is appreciable. The duty cycle for the advisory signs is observed to vary between 26 percent and 29 percent† with the higher duty cycles associated with the runs having larger values of acceleration noise and stopped time.

* The percent change calculations in the tables of this chapter tend to de-emphasize the improvements due to sign control. A reduction in acceleration noise from 10 to 5 is a 50% decrease whereas a change from 5 to 10 is a 100% increase.

† Since the vehicles travel at constant velocity before the lead vehicle maneuver begins, several signs near the beginning of the roadway are never set. Omitting these signs from the duty cycle calculation would increase the stated percents by about 25%.

Table 5-2, for $D = 2200$, similarly shows large reductions in stopped time and acceleration noise for both maneuvers and for all driver samples. However the improvements are not as dramatic as those for $D = 2300$ and there exists greater variability among the four driver samples.

Referring to Tables 5-3 through 5-8 it is seen that the stopped time is small for $D = 2100$ and essentially zero for smaller values of D both with and without sign control.

At $D = 2100$ the average reduction of acceleration noise is still substantial except for driver sample 3 under maneuver 2 for which the reduction is negligible. In Table 5-6, for $D = 1800$, it is observed that although there is an overall improvement due to sign control, three of the eight runs yield a small increase in acceleration noise.

The system's performance for different values of D is now evaluated on the basis of the percent change in stopped time and acceleration noise averaged over all driver samples for each maneuver. The average stopped time and the average acceleration noise (with and without the signs) and the average percent change* of these quantities are shown in Table 5-9 and Table 5-10 for maneuvers 1 and 2 respectively. For each value of D and for both maneuvers the simulation was run with four different driver samples, except for those few cases noted in the

*The average percent change is the average of the percent changes for the different driver samples, not the percent change of the average quantities appearing in Tables 5-9 and 5-10.

tables where three driver samples were used.

For maneuver 1 (see Table 5-9) the decrease in stopped time is dramatic for $D = 2300$ and $D = 2200$ but essentially negligible for smaller values of D for which vehicles are not forced to stop. The decrease in acceleration noise is substantial for values of D between 2300 and 2000 and is judged to be significant for the other demand rates. The results for maneuver 2 (see Table 5-10) are similar except for the negligible reduction in acceleration noise for $D = 1800$ and $D = 1700$.

Comparison of the results for maneuvers 1 and 2 show that, with few exceptions, the entries in Table 5-9 are greater than the corresponding entries in Table 5-10. It is not surprising that the stopped time and acceleration noise are larger for maneuver 1 since it is the more severe maneuver. The fact that the average percent changes are larger for maneuver 1 leads to the conclusion that sign control is comparatively more effective for larger perturbations.

From Table 5-11 it is observed that higher values of D require a higher duty cycle. Maneuver 1 requires a higher duty cycle, on the average, than maneuver 2, although the opposite is true for $D = 2200$ and $D = 2100$.

The results presented above have been for 100% compliance, that is, when all drivers respond to the advisory sign settings. In this study it has been hypothesized that sign control improvements will not be substantially eroded under conditions of partial compliance since a driver who responds to a sign forces the following drivers to decelerate. Results for partial compliance are presented in the following discussion.

The results at various compliance levels, for maneuver 1, are presented in Tables 5-12 through 5-16 for demand rates between 2300 and 1900 vehicles per hour[†]. The improvements due to sign control at lower demands are not large enough to warrant simulation runs at intermediate compliance levels.

For $D = 2300$ and $D = 2200$ (see Tables 5-12 and 5-13) the stopped time is appreciable without sign control. Stopped time generally increases as the compliance level decreases, yet a considerable improvement is shown at the 20 percent level of compliance. At $D = 2100$ (see Table 5-14) the stopped time is small and its variation with compliance level follows no pattern. At lower values of D the stopped time is zero or negligible.

[†] The simulation runs without sign control and those with sign control at 100 percent compliance were made, at most of the demand rates, for each of four different driver samples. However the time required for each simulation run precluded generating results for all four driver samples at the other compliance levels.

Tables 5-12 through 5-16 also indicate how the acceleration noise changes as a function of compliance level. Table 5-12 shows that for $D = 2300$ there is little degradation of performance, even at the 20 percent level of compliance. At $D = 2200$ the compliance level has minimal effect until it falls below 40%. At lower values of demand the compliance level begins to have a significant effect at a value between 80 and 60 percent. These results are summarized in Table 5-17 which presents the percent change in acceleration noise from the condition of no sign control to the condition of sign control at various levels of compliance. These same results are graphically presented in Fig.5-2.

5-4 Recommendations for Further Study

Based on the models assumed for car following and sign following, and using the algorithm developed in Chapter 4, a significant improvement in system performance has been demonstrated by using advisory sign control. No refinements in the mathematical models can substitute for field tests. It is recommended that a prototype system be constructed and tested, in order to determine the actual level of improvements.

An important consideration for any traffic control system is whether it effects a reduction in travel time. On the simulated highway the travel time of vehicle 1 is unaffected by sign control and the travel times of the other vehicles are completely determined by their time headways at the highway terminus, being unaffected by the dynamics on intermediate sections. On an actual highway with entrances and exits at intermediate points, the travel time of an arbitrary vehicle is profoundly affected by the dynamics of the flow. For example, if a vehicle is approaching its exit and there is a temporary stoppage of traffic between this vehicle and the exit, then this stopped time directly adds to the vehicle's travel time. Thus it is expected that the ability of a sign control system to reduce stopped time in dense traffic will lead to travel time savings on an actual highway. The extent of this reduction in travel time must await a field test.

For major highway incidents such as accidents and disabled vehicles the detectors can readily provide rapid and precise location of the disruption enabling swifter corrective action and thereby decreasing the total delay incurred by upstream vehicles. In addition it is expected that the incidence of rear-end accidents under these circumstances will be substantially reduced due to the advance warning nature of the advisory signs.

Although a cost analysis of the proposed system has not been undertaken it is felt that the system is economically feasible. The Toronto intersection control system, including 1000 detectors, is projected to cost \$5 million in capital outlay. Since a 16 mile length of six lane highway would use 1000 detectors, the cost of advisory sign control is estimated to be about \$300,000 per mile. This is a small fraction of the cost of constructing a mile of highway in an urban area.

An accurate cost-effective analysis would involve building the system and comparing before-and-after statistics on travel time, throughput, and accidents.

Further investigations should include the effect of advisory sign and detector spacings on improvements afforded by the system. These spacings are directly related to the cost of the system.

Table 5-1 Results without sign control and with sign control at 100% compliance,
for D = 2300 veh/hr

Driver Sample	Maneuver Number	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²			Duty Cycle with Sign Control (%)
		Without Signs	With Signs	Percent Change	Without Signs	With Signs	Percent Change	
1	1	328.	21.2	-93.6	38.92	14.11	-63.7	27.2
3	1	321.	18.8	-91.5	28.33	14.71	-48.2	25.2
4	1	707.	94.8	-86.6	40.85	17.58	-57.0	29.2
1	2	165.	27.8	-83.1	26.09	13.81	-47.2	25.9
3	2	199.	62.0	-68.9	26.42	14.10	-46.6	26.7
4	2	462.	49.6	-89.3	36.48	16.34	-55.2	27.6

Table 5-2 Results without sign control and with sign control at 100% compliance,
for D = 2200 veh/hr

Driver Sample	Maneuver Number	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²			Duty Cycle with Sign Control (%)
		Without Signs	With Signs	Percent Change	Without Signs	With Signs	Percent Change	
1	1	58.2	9.6	-83.5	18.64	11.49	-38.4	24.5
2	1	372.	64.4	-82.7	25.13	13.99	-44.3	26.9
3	1	46.4	1.0	-97.8	16.80	11.60	-30.9	24.6
4	1	342.	84.6	-75.3	33.91	13.70	-59.6	26.5
1	2	9.8	1.0	-89.8	13.61	11.19	-17.8	24.1
2	2	407.	51.4	-87.4	32.90	13.84	-58.0	27.1
3	2	32.8	11.6	-64.6	15.28	11.61	-24.0	25.4
4	2	64.2	31.6	-50.8	18.32	13.30	-27.4	26.9

Table 5-3 Results without sign control and with sign control at 100% compliance,
for D = 2100 veh/hr

Driver Sample	Maneuver Number	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²			Duty Cycle with Sign Control (%)
		Without Signs	With Signs	Percent Change †	Without Signs	With Signs	Percent Change	
1	1	0.0	2.8	-	10.51	8.742	-16.8	21.8
2	1	20.6	34.0	+65.0	16.91	10.87	-35.7	23.9
3	1	1.2	3.0	-	10.67	9.214	-13.6	22.1
4	1	41.4	19.6	-52.8	14.86	10.81	-27.3	24.6
1	2	0.0	1.2	-	9.234	8.026	-13.1	21.0
2	2	61.0	19.0	-68.9	19.40	11.052	-43.0	26.6
3	2	0.8	2.4	-	10.06	9.856	- 2.03	21.1
4	2	0.0	3.6	-	13.26	10.77	-18.8	28.1

† Missing entries in this column indicate that the change is zero or negligible and is treated as zero.

Table 5-4 Results without sign control and with sign control at 100% compliance,
for D = 2000 veh/hr

Driver Sample	Maneuver Number	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²			Duty Cycle with Sign Control (%)
		Without Signs	With Signs	Percent Change †	Without Signs	With Signs	Percent Change	
1	1	0.0	0.0	-	7.564	6.554	-13.4	19.7
2	1	0.0	1.2	-	11.21	8.735	-22.0	22.4
3	1	0.0	0.0	-	8.581	8.060	-6.07	21.9
4	1	0.0	0.0	-	10.93	8.259	-24.4	22.3
1	2	0.0	0.0	-	6.583	6.217	-5.56	17.6
2	2	0.0	0.0	-	10.66	8.107	-23.9	22.3
3	2	0.0	0.0	-	8.182	7.038	-14.0	19.6
4	2	0.0	0.0	-	10.15	8.120	-20.0	24.0

† Missing entries in this column indicate that the change is zero or negligible and is treated as zero.

Table 5-5 Results without sign control and with sign control at 100% compliance,

for $D = 1900$ veh/hr

Driver Sample	Maneuver Number	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²			Duty Cycle with Sign Control (%)
		Without Signs	With Signs	Percent Change	Without Signs	With Signs	Percent Change	
1	1	0.0	0.0	-	5.697	5.517	-3.16	18.1
2	1	0.0	0.0	-	7.843	6.974	-11.1	18.1
3	1	0.0	0.0	-	6.648	6.006	-9.66	20.1
4	1	0.0	0.0	-	7.158	6.731	-5.97	20.6
1	2	0.0	0.0	-	5.079	4.807	-5.36	15.1
2	2	0.0	0.0	-	7.299	6.306	-13.6	20.4
3	2	0.0	0.0	-	6.581	5.798	-11.9	19.5
4	2	0.0	0.0	-	7.027	6.234	-11.3	19.0

Table 5-6 Results without sign control and with sign control at 100% compliance,
for D = 1800 veh/hr

Driver Sample	Maneuver Number	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²			Duty Cycle with Sign Control (%)
		Without Signs	With Signs	Percent Change	Without Signs	With Signs	Percent Change	
1	1	0.0	0.0	-	4.791	4.324	-9.75	17.2
2	1	0.0	0.0	-	5.851	6.199	+5.95	18.5
3	1	0.0	0.0	-	5.618	5.180	-7.79	16.7
4	1	0.0	0.0	-	5.603	4.939	-11.9	17.1
1	2	0.0	0.0	-	4.288	3.909	-8.84	13.2
2	2	0.0	0.0	-	5.552	5.734	+3.28	17.7
3	2	0.0	0.0	-	5.382	5.031	-6.52	16.1
4	2	0.0	0.0	-	4.851	5.009	+3.26	15.6

Table 5-7 Results without sign control and with sign control at 100% compliance,
for D = 1700 veh/hr

Driver Sample	Maneuver Number	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²			Duty Cycle with Sign Control (%)
		Without Signs	With Signs	Percent Change	Without Signs	With Signs	Percent Change	
1	1	0.0	0.0	-	3.962	3.494	-11.8	15.1
2	1	0.0	0.0	-	4.911	4.917	+0.1	14.1
3	1	0.0	0.0	-	4.814	4.321	-10.2	16.7
4	1	0.0	0.0	-	4.271	4.291	+0.5	15.5
1	2	0.0	0.0	-	3.542	3.309	-6.58	13.4
2	2	0.0	0.0	-	4.654	4.803	+3.2	16.5
4	2	0.0	0.0	-	4.007	3.980	-0.7	15.5

Table 5-8 Results without sign control and with sign control at 100% compliance,
for D = 1600 veh/hr

Driver Sample	Maneuver Number	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²			Duty Cycle with Sign Control (%)
		Without Signs	With Signs	Percent Change	Without Signs	With Signs	Percent Change	
1	1	0.0	0.0	-	3.240	2.787	-14.0	12.9
2	1	0.0	0.0	-	4.386	4.146	-5.5	15.7
4	1	0.0	0.0	-	3.629	3.364	-7.3	14.6

Table 5-9 Results without sign control and with sign control at 100% compliance, averaged over several driver samples, for maneuver 1

Demand D (veh/hr)	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²		
	Average Without Signs	Average With Signs	Average Percent Change [†]	Average Without Signs	Average With Signs	Average Percent Change
2300*	418.7	44.9	-90.5	36.03	15.46	-56.3
2200	204.7	39.9	-84.8	23.62	12.70	-43.3
2100	15.8	14.9	-	13.24	9.91	-23.4
2000	0.0	0.3	-	9.57	7.90	-16.5
1900	0.0	0.0	-	6.84	6.31	-7.5
1800	0.0	0.0	-	5.47	5.16	-5.9
1700	0.0	0.0	-	4.49	4.26	-5.4
1600**	0.0	0.0	-	3.75	3.43	-8.9

* Excludes driver sample 2

** Excludes driver sample 3

† The missing entries in this column indicate that the change is zero or negligible and is treated as zero.

Table 5-10 Results without sign control and with sign control at 100% compliance, averaged over several driver samples, for maneuver 2

Demand D (veh/hr)	Stopped Time (sec)			Acceleration Noise (ft/sec ²) ²		
	Average Without Signs	Average With Signs	Average Percent Change †	Average Without Signs	Average With Signs	Average Percent Change
2300*	275.3	4.65	-80.4	29.66	14.75	-49.6
2200	128.4	23.9	-73.2	20.03	12.49	-31.8
2100	15.45	6.55	-17.2	12.99	9.93	-19.2
2000	0.0	0.0	-	8.89	7.37	-15.9
1900	0.0	0.0	-	6.497	5.786	-10.5
1800	0.0	0.0	-	5.018	4.921	-2.2
1700**	0.0	0.0	-	4.067	4.031	-1.4

* Excludes driver sample 2

** Excludes driver sample 3

† Missing entries in this column indicate that the change is zero

Table 5-11 Average duty cycle of advisory signs at 100% compliance

Demand D (veh/hr)	Duty Cycle (%)	
	Maneuver 1	Maneuver 2 *
2300	27.2	26.7
2200	25.6	25.9
2100	23.1	24.2
2000	21.6	20.9
1900	19.2	18.5
1800	17.4	15.7
1700	15.4	15.1
1600	14.4	-

* Maneuver 2 was not run for D = 1600

**Table 5-12 Results at various compliance levels,
for maneuver 1, for D = 2300[†]**

Compliance Level (%)	Stopped Time (sec)	Acceleration Noise (ft/sec ²) ²	Duty Cycle (%)
100	44.9	15.46	27.2
80	82.7	15.93	27.1
60	76.3	15.72	29.2
40	74.4	16.02	28.1
20	106.6	18.53	29.5
0*	418.6	36.03	-

[†] Includes driver samples 1, 3, and 4

* Results for no sign control

**Table 5-13 Results at various compliance levels,
for maneuver 1, for D = 2200[†]**

Compliance Level (%)	Stopped Time (sec)	Acceleration Noise (ft/sec ²) ²	Duty Cycle (%)
100	52.9	13.06	26.0
80	54.2	13.70	27.4
60	73.2	13.62	28.4
40	98.5	14.70	29.9
20	117.	16.81	29.6
0*	257.	25.90	-

[†] Includes driver samples 1, 2, and 4

* Results for no sign control

Table 5-14 Results at various compliance levels,
for maneuver 1, for D = 2100[†]

Compliance Level (%)	Stopped Time (sec)	Acceleration Noise (ft/sec ²) ²	Duty Cycle (%)
100	18.5	10.04	23.0
80	6.3	10.46	24.5
60	28.2	11.36	25.0
40	35.7	11.58	25.3
20	3.1	12.24	25.4
0*	10.9	13.79	-

[†] Includes driver samples 2 and 3

* Results for no sign control

Table 5-15 Results at various compliance levels,
for maneuver 1, for D = 2000[†]

Compliance Level (%)	Stopped Time (sec)	Acceleration Noise (ft/sec ²) ²	Duty Cycle (%)
100	0.4	7.62	21.3
80	1.1	7.89	20.9
60	0.0	7.86	22.4
40	0.0	8.15	20.7
20	1.9	8.28	21.8
0*	0.0	9.02	-

[†] Includes driver samples 1, 3, and 4

* Results for no sign control

Table 5-16 Results at various compliance levels,
for maneuver 1, for D = 1900[†]

Compliance Level (%)	Stopped Time (sec)	Acceleration Noise (ft/sec ²) ²	Duty Cycle (%)
100	0.0	6.41	18.9
80	0.0	6.50	19.5
60	0.0	6.69	19.0
40	0.0	6.65	19.2
20	0.0	6.59	20.3
0*	0.0	6.90	-

[†]Includes driver samples 1,2, and 4

*Results for no sign control

Table 5-17 Percent change in acceleration noise due to
sign control at various compliance levels,
for maneuver 1

Demand D (veh/hr)	Percent Change in Acceleration Noise				
	Compliance Level				
	100%	80%	60%	40%	20%
2300	-57	-57	-58	-57	-50
2200	-50	-47	-47	-43	-35
2100	-27	-24	-18	-16	-11
2000	-16	-13	-13	-10	- 8
1900	- 7	- 6	- 3	- 4	- 4

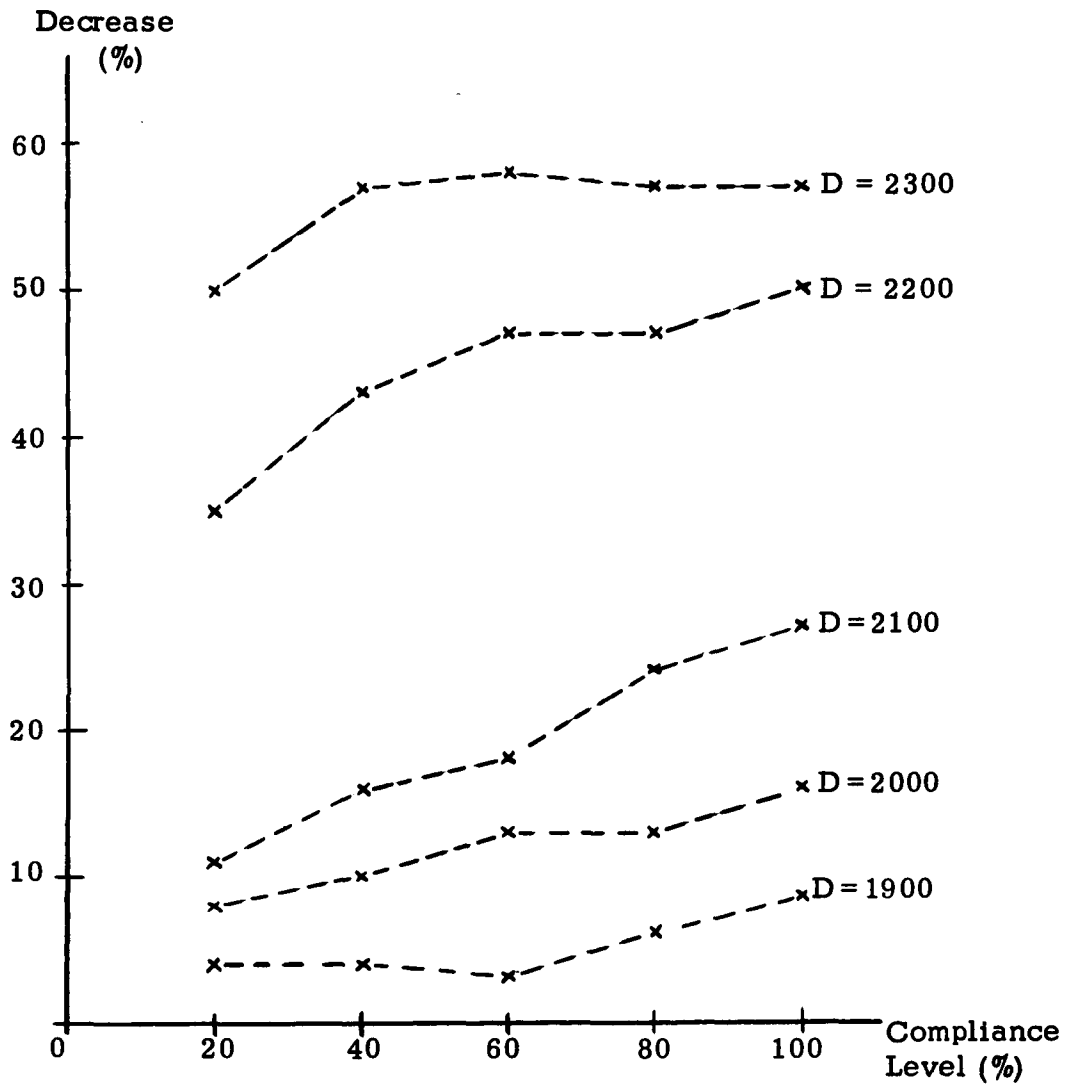


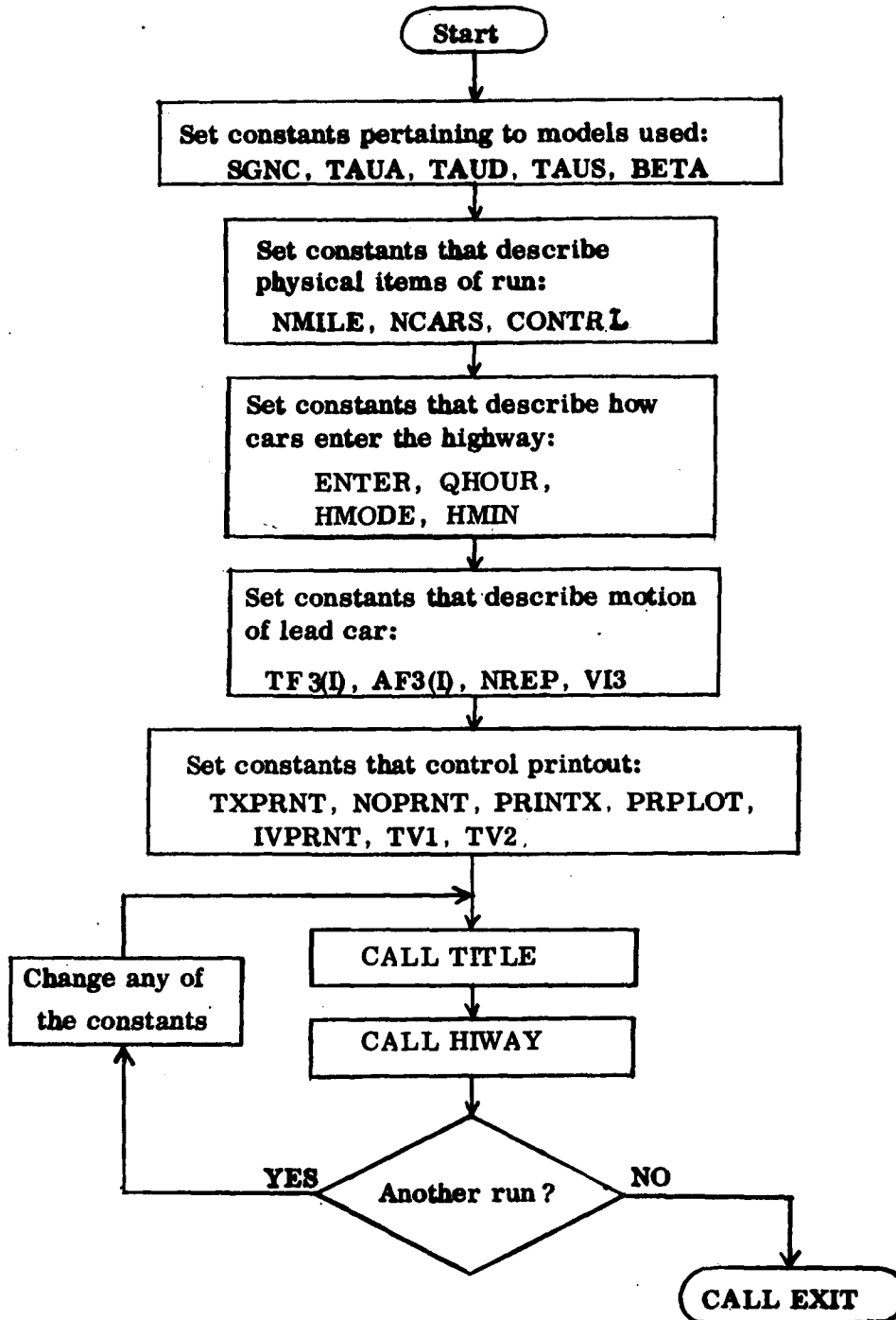
Fig. 5-2 Percent decrease in acceleration noise due to sign control at various compliance levels, for maneuver 1

APPENDIX A

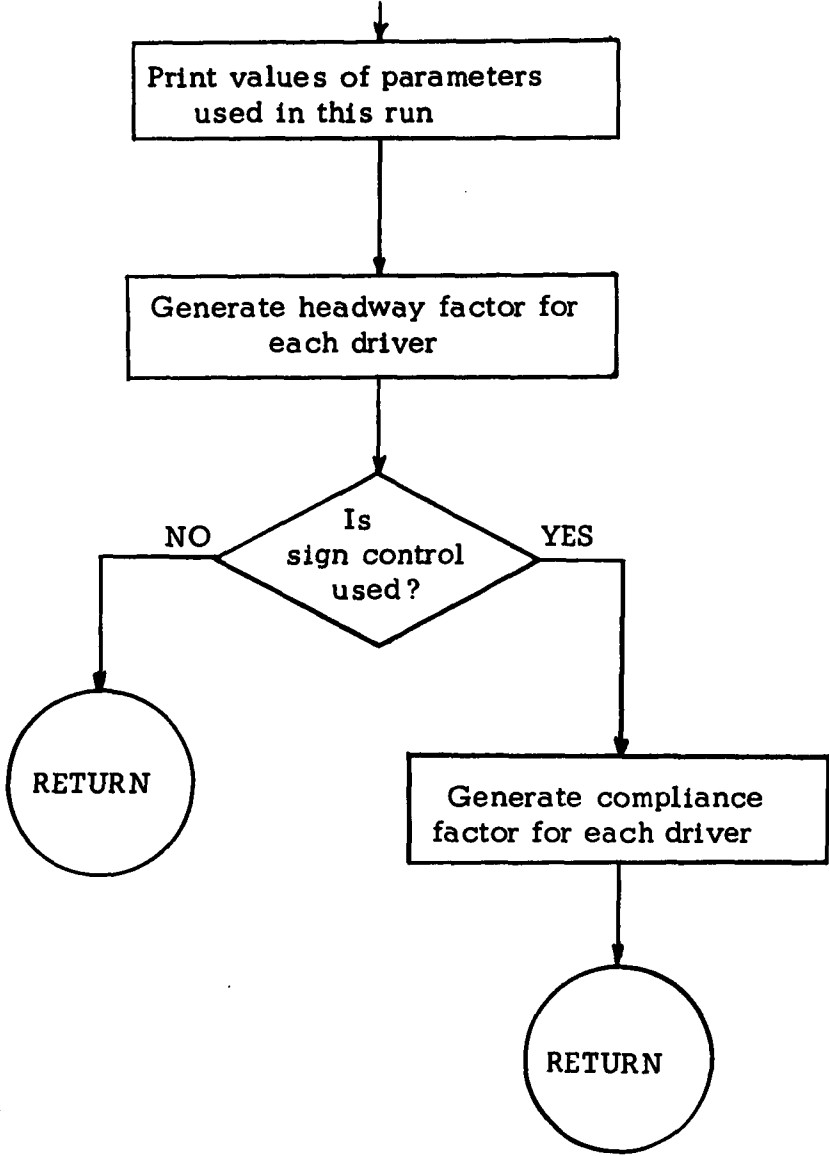
FLOW CHARTS OF THE COMPUTER

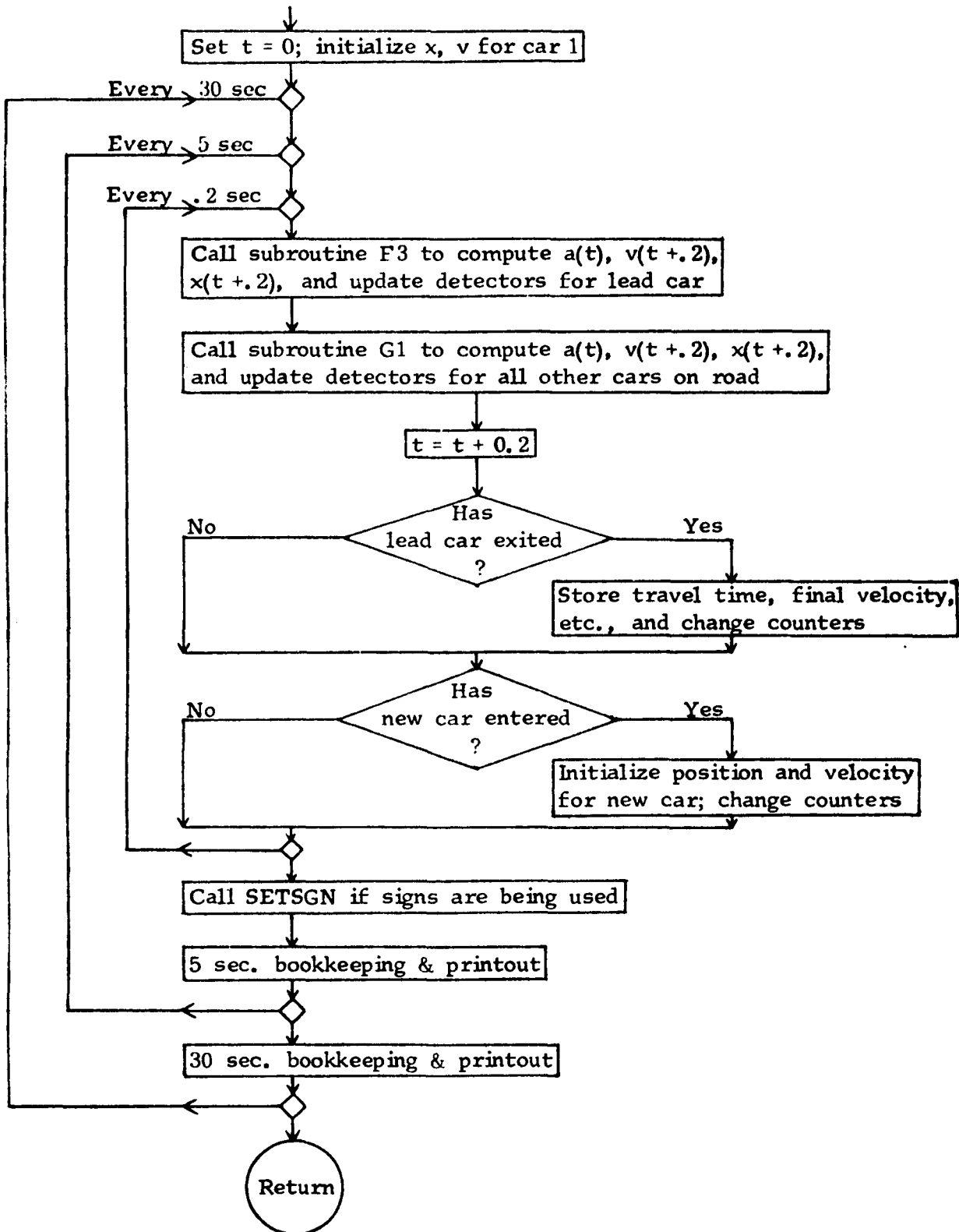
SIMULATION PROGRAM

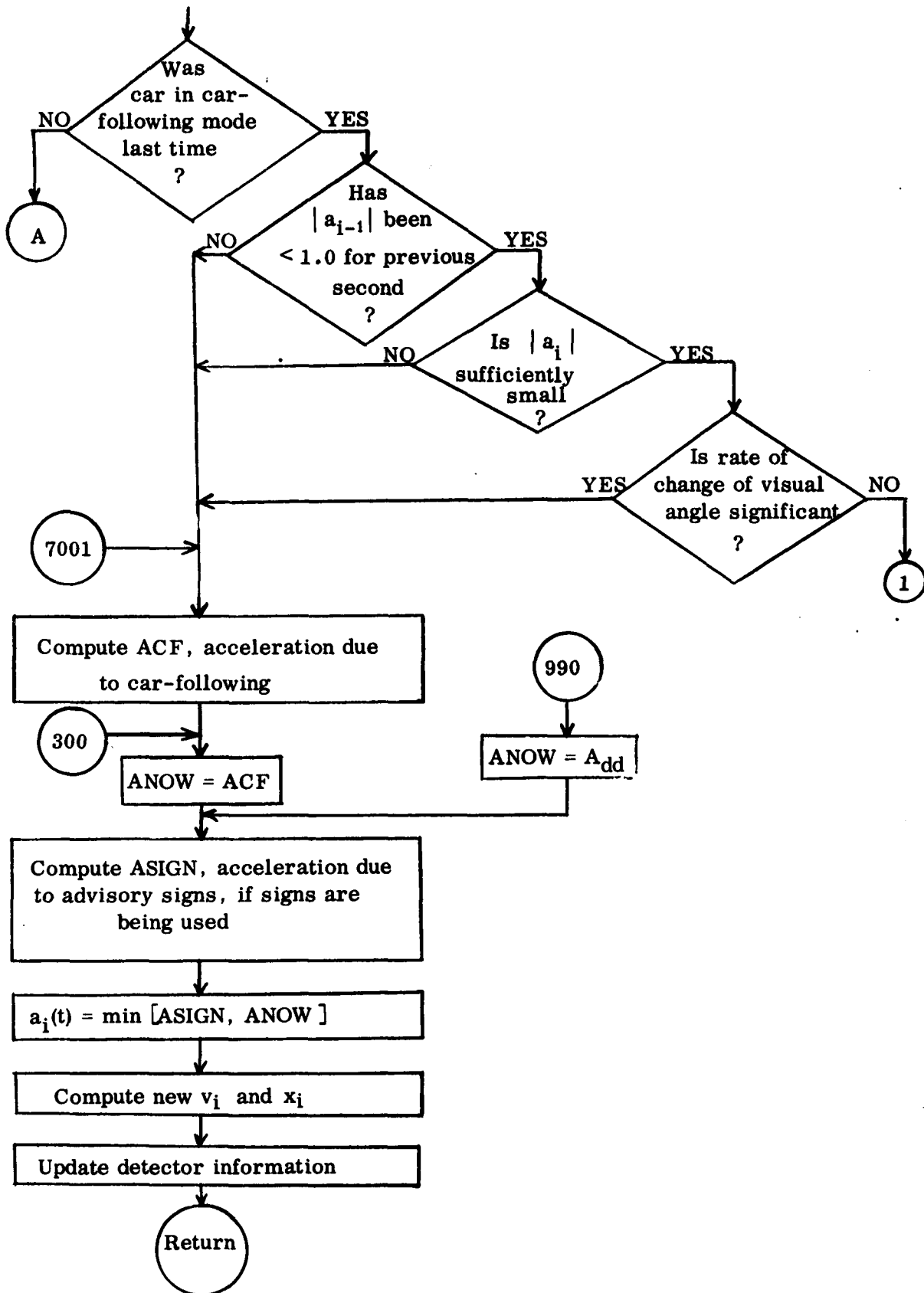
Main Program



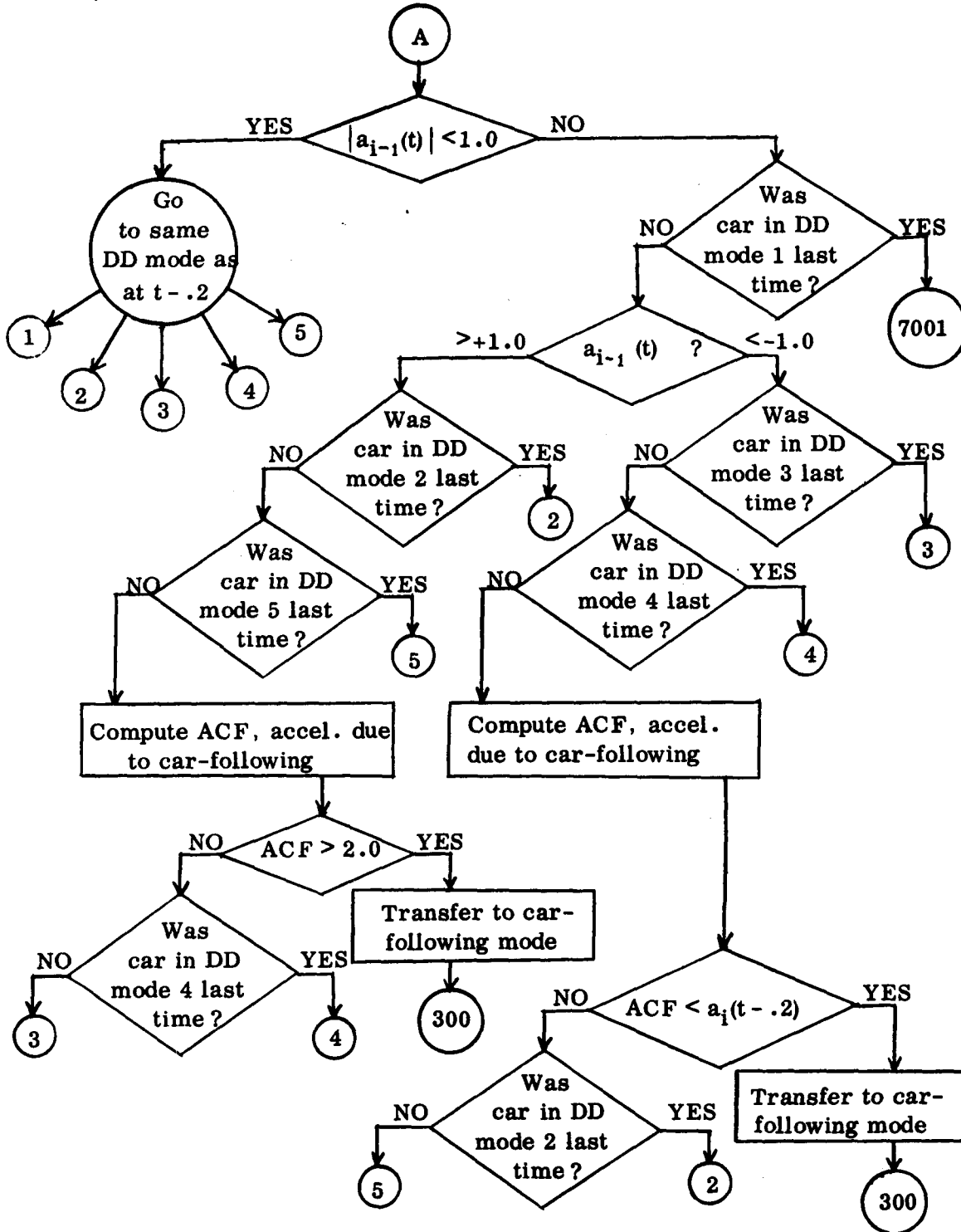
Subroutine TITLE



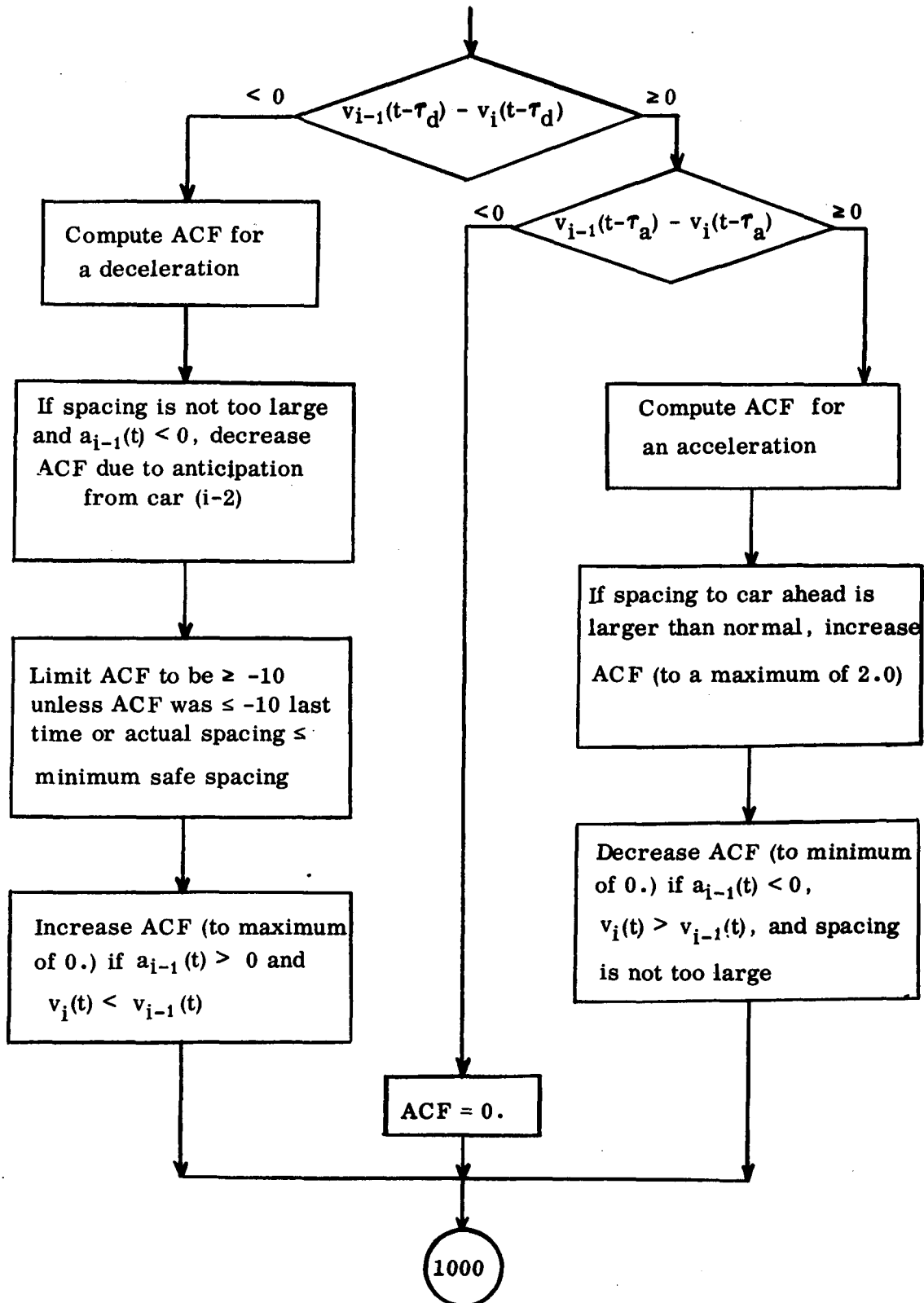




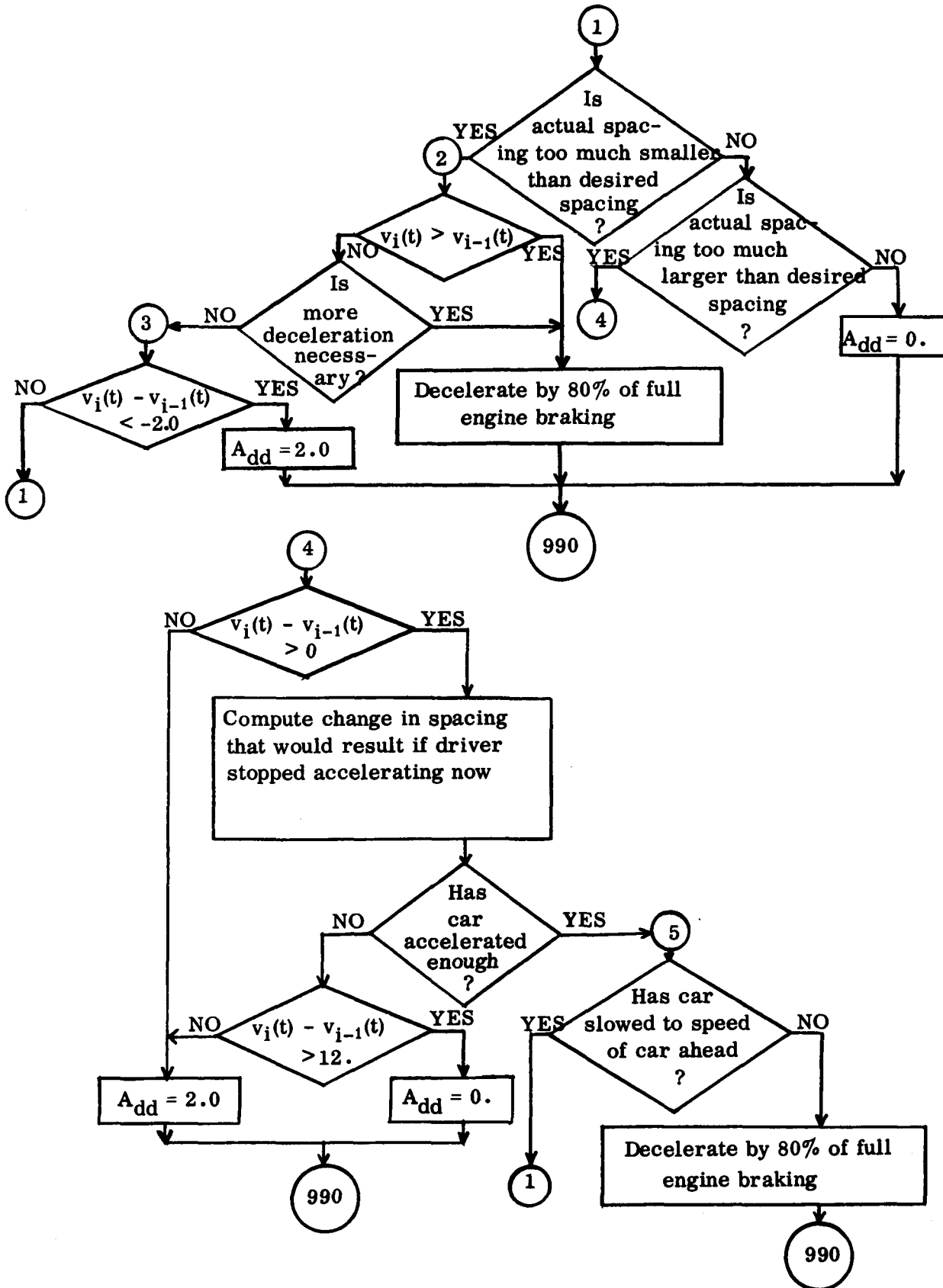
Subroutine G1 --- Vehicle Dynamics



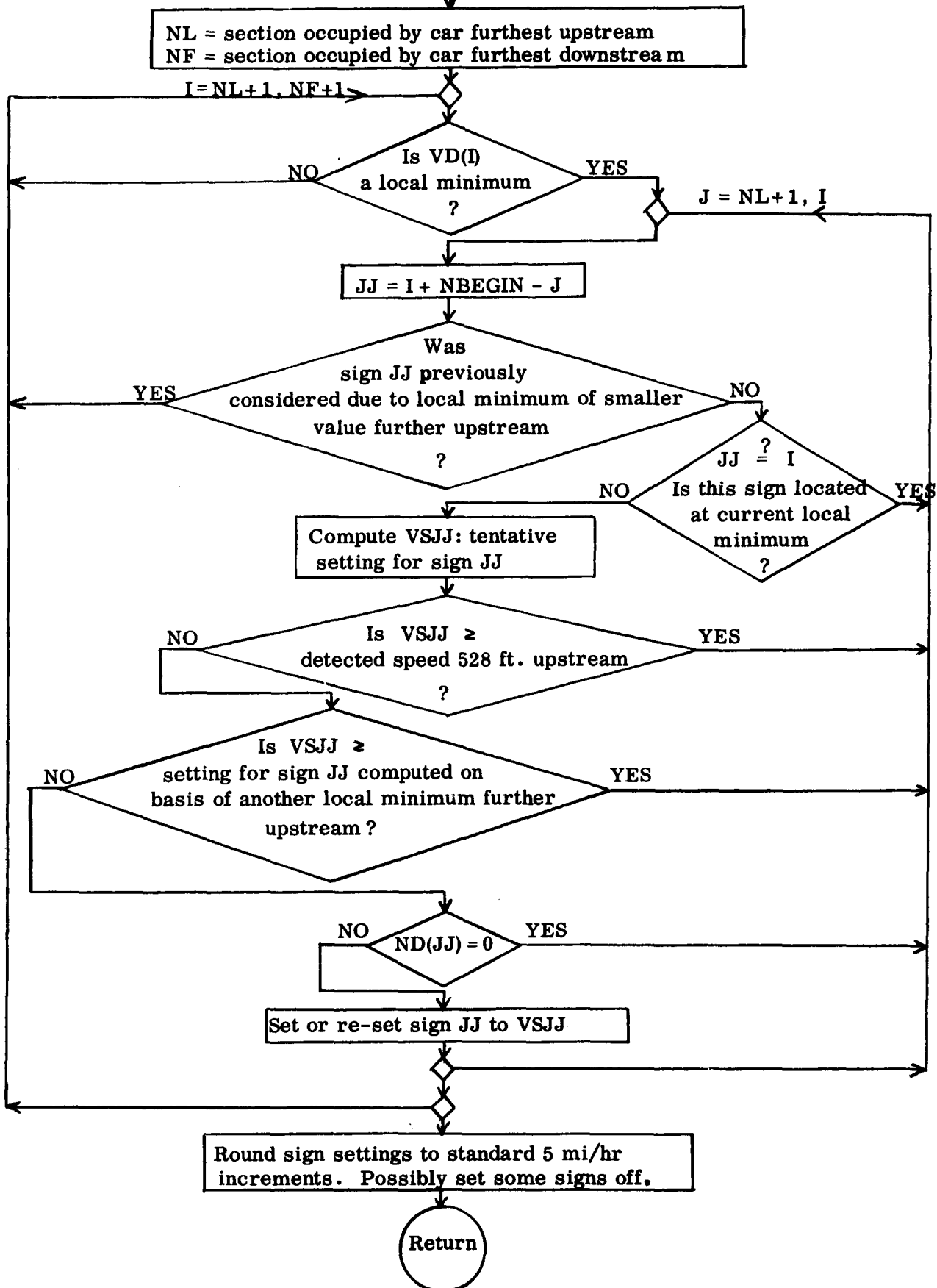
Subroutine G1 --- Acceleration Due to Car-following (ACF)



Subroutine G1 --- Distance-Detection Mode (DD mode)

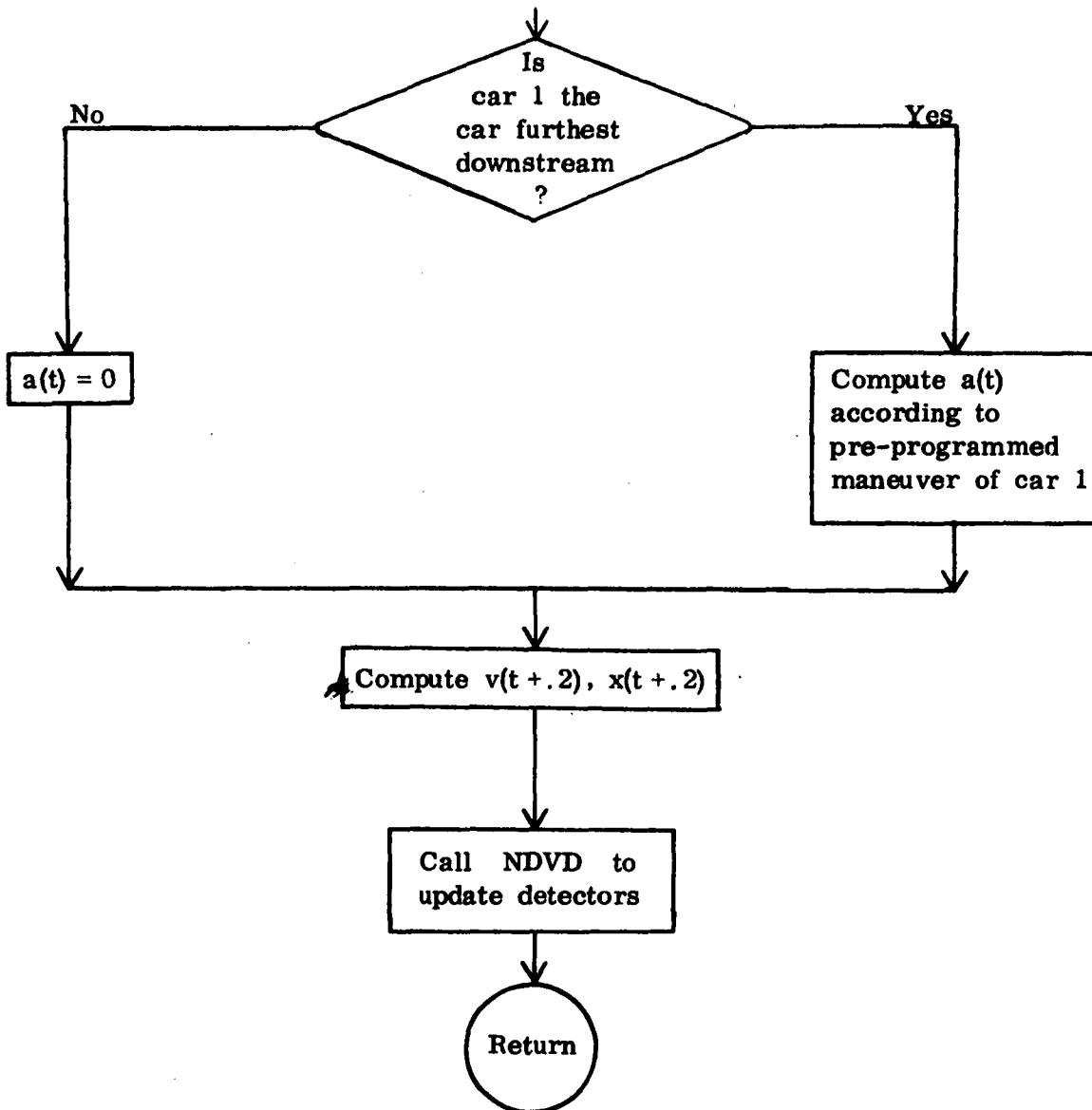


Subroutine SETSGN -- Computes Advisory Sign Settings



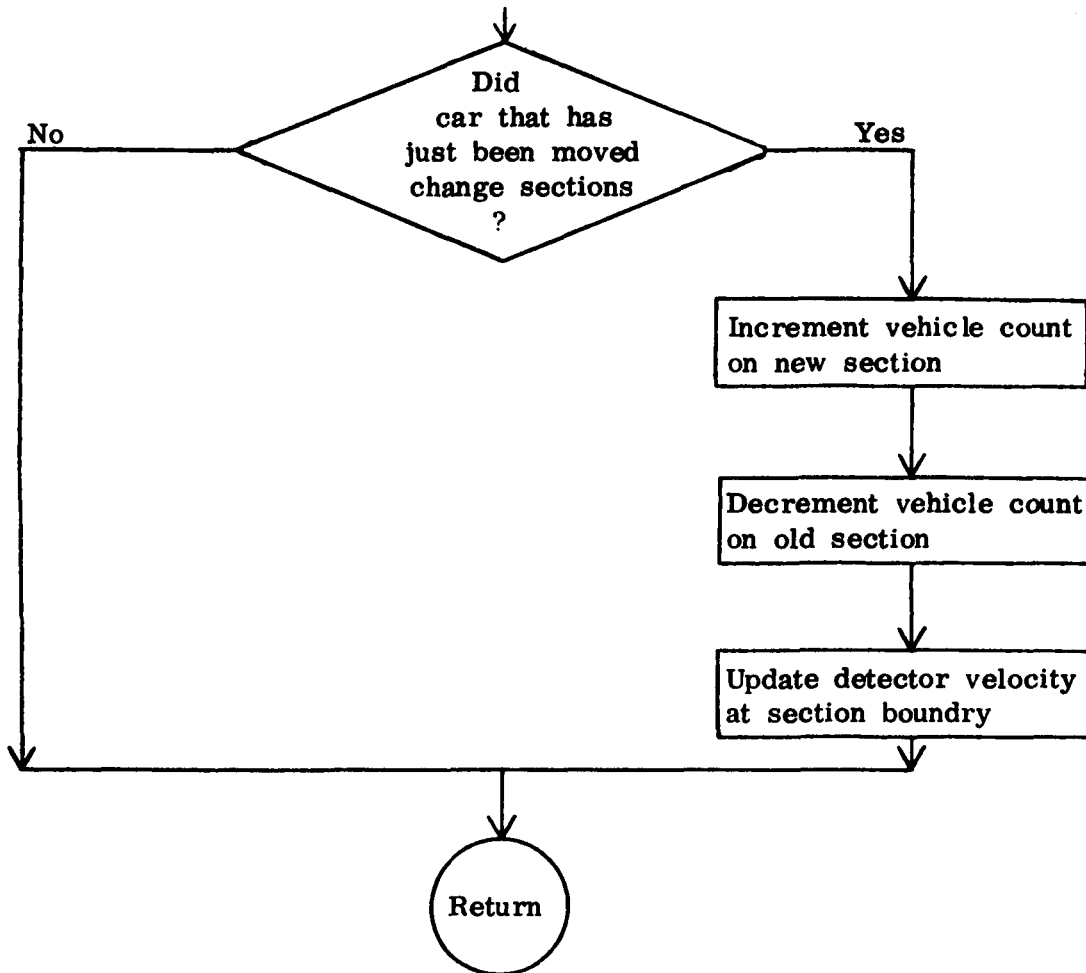
Subroutine F3

This subroutine updates velocity and position for the car that is furthest downstream



Subroutine NDVD

This subroutine updates ND and VD -- Detector Information



APPENDIX B

LISTING OF FORTRAN SIMULATION

PROGRAM

C	COMMONS USED	MAIN	10
C		MAIN	20
C	MAIN ABC HM T ZK	MAIN	30
C	TITLE ABC G T ZK	MAIN	40
C	HIWAY \$A C FGHMN U Z	MAIN	50
C	G1 \$A G MN T	MAIN	60
C	F3 \$AB G	MAIN	70
C	SETSGN \$A C N U	MAIN	80
C	NDVD \$ G N	MAIN	90
C	FINISH ABC F MN U ZK	MAIN	100
C	ZAP M ZK	MAIN	110
C		MAIN	120
	IMPLICIT INTEGER(C,I-N)	MAIN	130
	INTEGER*2 IBRNCH, IDMODE, COMPLY	MAIN	140
	INTEGER ON, OFF, ENTER, PRINTX	MAIN	150
	COMMON /AAAAAA/ NCARS, NMILE, VSTART, TXPRNT, IBRNCH(350),	MAIN	160
	1 CARNOW, NOPRNT, CONTRL, ON, OFF, BETA, TAUS, KSIGN, L SIGN(100),	MAIN	170
	2 HBAR, HDRV(350), COMPLY(350), VSIGN(100,7)	MAIN	180
	COMMON /BBBBBB/ TF3(5), AF3(4), BASE, NREP, NREPI, VI3	MAIN	190
	COMMON /CCCCCC/ CBUG1, CROUND, SGNC	MAIN	200
	COMMON /HHHHHH/ PRINTX, BOMB, TXPR2, NOPR2	MAIN	210
	COMMON /KKKKKK/ ESIG2, ETSTOP, ETOTLT, EPER, EPCMP, IEVAV1, IEDX, NENS,	MAIN	220
	2 JENS, EQ, ESPACE(10), RRI	MAIN	230
	COMMON /MMMMMM/ I VPRNT, TV1, TV2, IFIFTH, IFOL, SPACE(10), TSPACE(10)	MAIN	240
	COMMON /TTTTTT/ IDMODE(350), TAU, TAUD, VLIMIT, KA, KD	MAIN	250
	COMMON /ZZZZZZ/ ENTER, QHOUR, HMIN, HMCDE, IR, HBAR2, PCMP, PCMP	MAIN	260
	DATA ION, IOFF / 'ON', 'OFF' /	MAIN	270
	ON = ION	MAIN	280
	OFF = IOFF	MAIN	290
C		MAIN	300
C	CBUG1 SPECIFIES PRINTOUT OF DETAILS OF SIGN SETTING	MAIN	310
C	CBUG1=0 NO PRINTOUT	MAIN	320
C	CBUG1=1 PRINTOUT EVERY 5 SECONDS	MAIN	330
C		MAIN	340
	CBUG1 = 0	MAIN	350
C		MAIN	360
C	CROUND SPECIFIES ROUNDING OF SIGN SETTINGS	MAIN	370

C	CROUND=0	EXACT SIGN SETTINGS USED	MAIN	380
C	CROUND=1	ROUND SIGN SETTINGS -- USE MODEL 1	MAIN	390
C			MAIN	400
C			MAIN	410
C	PCMPLY =	FRACTION OF DRIVERS THAT COMPLY WITH SIGNS	MAIN	420
C			MAIN	430
C	PCMPLY =	1.0	MAIN	440
C			MAIN	450
C			MAIN	460
C	SGNC IS A	CONSTANT IN ADJUSTING SIGN SETTING	MAIN	470
C			MAIN	480
C	SGNC =	1200.	MAIN	490
C			MAIN	500
C	*****	INPUT DATA	MAIN	510
C			MAIN	520
C	HDRV(I)	= VARIABLE AFFECTING DESIRED DISTANCE	MAIN	530
C	NCARS	= TOTAL # OF CARS TO ENTER THE HIGHWAY	MAIN	540
C	NMILE	= LENGTH OF HIGHWAY (IN MILES)	MAIN	550
C	BETA	= SENSITIVITY CONST. TO COMPUTE ACC. DUE TO SIGNS	MAIN	560
C	TAUS	= DELAY IN RESPONSE DUE TO SIGNS	MAIN	570
C		KSIGN = 5.*TAUS + .1 + 1. (SET IN HIWAY)	MAIN	580
C			MAIN	590
C	BETA =	.2	MAIN	600
C	TAUS =	2.0	MAIN	610
C	TAUA =	1.4	MAIN	620
C	TAUD=	1.0	MAIN	630
C			MAIN	640
C	SET	CONSTANTS TO SPECIFY DENSITY FUNCTION FOR HEADWAY FACTOR	MAIN	650
C	HMIN	= MINIMUM HEADWAY FACTOR	MAIN	660
C	HMODE	= MODAL HEADWAY FACTOR	MAIN	670
C			MAIN	680
C	HMIN =	.3	MAIN	690
C	HMODE =	1.	MAIN	700
C			MAIN	710
C			MAIN	720
C	QHOUR =	AVG. ENTERING FLOW RATE (CARS/HOUR) FOR	MAIN	730
C		RANDOM ENTERING HEADWAYS	MAIN	740

C		MAIN 750
	QHOOR = 2200.	MAIN 760
C		MAIN 770
C	ENTER = 0 IF ENTERING SPACING IS SAME FOR ALL CARS	MAIN 780
C	= 1 FOR DISTRIBUTION 1 (SEE TITLE)	MAIN 790
C		MAIN 800
C		MAIN 810
C	DXCAR, VCAR, ANOW, IMODE ARE PRINTED OUT ONCE EVERY	MAIN 820
C	'NOPRNT' CYCLES (AFTER TNCW = TXPRNT)	MAIN 830
C		MAIN 840
C	IF 'PRINTX=1' THEN XCAR ALSO PRINTED OUT	MAIN 850
C		MAIN 860
C	IVPRNT CONTROLS PRINTING OF VELOCITY OF CARS EVERY SECOND	MAIN 870
C	IVPRNT = 0 VELOCITIES NOT PRINTED	MAIN 880
C	IVPRNT = 1 VELOCITIES PRINTED	MAIN 890
C	TV1 = TIME TO START PRINTING VELOCITIES	MAIN 900
C	TV2 = TIME TO STOP PRINTING VELOCITIES	MAIN 910
C		MAIN 920
C	CONTRL = 0 IF NO SIGN CONTROL	MAIN 930
C	= 1 WITH SIGN CONTROL	MAIN 940
C		MAIN 950
C		MAIN 960
C	NENS SPECIFIES NUMBER OF ENSEMBLE RUNS (MAX OF 10)	MAIN 970
C		MAIN 980
	NENS = 1	MAIN 990
	NENS = 4	MAIN 1000
C		MAIN 1010
C	IF YOU WISH RUN TO END AFTER 'X' TRAVEL SECONDS,	MAIN 1020
C	THEN . . . 'BOMB=X'	MAIN 1030
C	OTHERWISE... 'BOMB=1000000.'	MAIN 1040
C		MAIN 1050
C		MAIN 1060
C	TXPRNT = TIME WHEN PRINTING OF X,V,A,MODE BEGINS	MAIN 1070
C	IF YOU DO NOT WISH TO HAVE X, V, A, MODE	MAIN 1080
C	PRINTED OUT AT ALL, SET TXPRNT = 1000000.	MAIN 1090
C	BEFORE CALLING HIWAY	MAIN 1100
C		MAIN 1110

C	TXPR2 = TIME TO CHANGE TO PRINTING X,V,A,MODE EVERY 'NOPR2'	MAIN 1120
C	CYCLES	MAIN 1130
C		MAIN 1140
	TXPRNT=1.E6	MAIN 1150
	TXPR2 = 1.E6	MAIN 1160
C		MAIN 1170
C		MAIN 1180
C	IF USING F3 FOR LEAD CAR MANEUVER, SET	MAIN 1190
C	NREP = NUMBER OF REPETITIONS OF BASIC MANEUVER	MAIN 1200
C	TF3(I) = TIME INSTANTS DESCRIBING BASIC MANEUVER	MAIN 1210
C	AF3(I) = ACCELERATIONS USED IN BASIC MANEUVER (FT/SEC/SEC)	MAIN 1220
C	VI3 = INITIAL VELOCITY (MI/HR)	MAIN 1230
C		MAIN 1240
	NOPRNT = 1	MAIN 1250
	NOPR2 = 5	MAIN 1260
	PRINTX = 0	MAIN 1270
	TV1 = .220.	MAIN 1280
	TV2 = 2000.	MAIN 1290
	BOMB = 2000.	MAIN 1300
	TF3(1) = 0. + 237.	MAIN 1310
	TF3(2) = 7.4 + 237.	MAIN 1320
	TF3(3) = 27.4 + 237.	MAIN 1330
	TF3(4) = 34.8 + 237.	MAIN 1340
	TF3(5) = 54.8 + 237.	MAIN 1350
	AF3(1) = -3.0	MAIN 1360
	AF3(2) = 0.0	MAIN 1370
	AF3(3) = 3.0	MAIN 1380
	AF3(4) = 0.	MAIN 1390
	NREP = 50	MAIN 1400
	VI3 = 50.	MAIN 1410
	ENTER = 1	MAIN 1420
	CROUND = 1	MAIN 1430
	NMILE = 10	MAIN 1440
	NCARS = 80	MAIN 1450
	IVPRNT = 0	MAIN 1460
	NENS = 4	MAIN 1470
C		MAIN 1480

C	QHOOR VARIES, PCMPY = 1.0	MAIN 1490
C	QHOOR = 2000.	MAIN 1500
	CONTRL = 0	MAIN 1510
	CALL ZAP	MAIN 1520
	CONTRL = 1	MAIN 1530
	CALL ZAP	MAIN 1540
C	QHOOR = 1900.	MAIN 1550
	CONTRL = 0	MAIN 1560
	IVPRNT = 1	MAIN 1570
	CALL ZAP	MAIN 1580
	CONTRL = 1	MAIN 1590
	CALL ZAP	MAIN 1600
C	QHOOR = 1800.	MAIN 1610
	CONTRL = 0	MAIN 1620
	IVPRNT = 0	MAIN 1630
	CALL ZAP	MAIN 1640
	CONTRL = 1	MAIN 1650
	CALL ZAP	MAIN 1660
C	QHOOR = 1700.	MAIN 1670
	CONTRL = 0	MAIN 1680
	CALL ZAP	MAIN 1690
	CONTRL = 1	MAIN 1700
	CALL ZAP	MAIN 1710
C	QHOOR = 1600.	MAIN 1720
	CONTRL = 0	MAIN 1730
	IVPRNT = 1	MAIN 1740
	CALL ZAP	MAIN 1750
	CONTRL = 1	MAIN 1760
	CALL ZAP	MAIN 1770
C	QHOOR = 1500.	MAIN 1780
	IVPRNT = 0	MAIN 1790
		MAIN 1800
		MAIN 1810
		MAIN 1820
		MAIN 1830
		MAIN 1840
		MAIN 1850

CONTRL = 0
CALL ZAP
CONTRL = 1
CALL ZAP
C
QHOOR = 1400.
CONTRL = 0
CALL ZAP
CONTRL = 1
CALL ZAP
C
QHOOR = 1300.
IVPRNT = 1
CONTRL = 0
CALL ZAP
CONTRL = 1
CALL ZAP
C
CALL EXIT
STOP
END

MAIN 1860
MAIN 1870
MAIN 1880
MAIN 1890
MAIN 1900
MAIN 1910
MAIN 1920
MAIN 1930
MAIN 1940
MAIN 1950
MAIN 1960
MAIN 1970
MAIN 1980
MAIN 1990
MAIN 2000
MAIN 2010
MAIN 2020
MAIN 2030
MAIN 2040
MAIN 2050
MAIN 2060

```

SUBROUTINE HIWAY
C
C ***** VARIABLES USED IN THIS PROGRAM (UNITS OF FEET & SECONDS)
C
C     TNOW      = TIME NOW
C     CARONE    = THE CAR THAT IS FARTHEST AHEAD ON THE HIGHWAY NOW
C     CYCLE1    = CARONE HAS POSITION CYCLE1 IN THE ARRAYS,
C     I.E. XCAR(CYCLE1,16)= WHERE FURTHEST CAR IS NOW
C     CARZ      = SAME AS ABOVE FOR LAST CAR ON THE ROAD
C     CYCLEZ    = SAME AS ABOVE FOR LAST CAR ON THE ROAD
C     CARA, CYCLEA REFER TO FURTHEST CAR ON HIGHWAY THAT HASN'T
C     REACHED THE POINT AT WHICH TRAVEL TIME IS RECORDED
C
C     JSTOP(I) = NUMBER OF INTERVALS I-TH CAR IS AT REST
C     ATOTAL(I) = SUM OF ANOW(I)**2
C
C     NFEET     = NMILE * 5280
C     TRAVEL(I) = 1. TIME WHEN I-TH CAR ENTERS HIGHWAY
C     2. TRAVELLING TIME OF I-TH CAR ON HIGHWAY
C
C     ANOW(I)   = LAST ACC OF I-TH CAR
C     JEXIT(I)  INDICATES WHEN I-TH CAR LEFT THE HIGHWAY
C
C     IMPLICIT INTEGER(C,I-N)
C     INTEGER*2 JEXIT, JSTOP, ZD(7), ZSIGN(7), COMPLY
C     INTEGER*2 ND, IVOUT(350), IDXOUT(350), IBRNCH, NSON, NSFULL, CECT
C     INTEGER*2 JSEC, JVSET, IVSEC(100,5)
C     INTEGER PRINTX, IX(20), ON, OFF, ENTER
C     DIMENSION IDX(20), VV(20)
C     COMMON XCAR(3861), VCAR(3861), ANOW(350), CYCLE1, CARONE, TNOW, CARA,
1 CARZ, CYCLEZ, CYCLEA
C     COMMON /AAAAAA/ NCARS, NMILE, VSTART, TXPRNT, IBRNCH(350),
1 CARNOW, NOPRNT, CONTRL, ON, OFF, BETA, TAUS, KSIGN, LSIGN(100),
2 HBAR, HDRV(350), COMPLY(350), VSIGN(100,7)
C     COMMON /CCCCC/ CBUG1, CROUND, SGNC
C     COMMON /FFFFFF/ TOTALT, ATOTAL(350), TRAVEL(350), JEXIT(350),
HIWA 10
HIWA 20
HIWA 30
HIWA 40
HIWA 50
HIWA 60
HIWA 70
HIWA 80
HIWA 90
HIWA 100
HIWA 110
HIWA 120
HIWA 130
HIWA 140
HIWA 150
HIWA 160
HIWA 170
HIWA 180
HIWA 190
HIWA 200
HIWA 210
HIWA 220
HIWA 230
HIWA 240
HIWA 250
HIWA 260
HIWA 270
HIWA 280
HIWA 290
HIWA 300
HIWA 310
HIWA 320
HIWA 330
HIWA 340
HIWA 350
HIWA 360
HIWA 370

```

1	JSTOP(350),IDXOUT,IVOUT	HIWA 380
	COMMON /GGGGGG/ ISIGN,JSEC,JVSET,IVSEC,IVP,ITENP,CECT(350)	HIWA 390
	COMMON /HHHHHH/ PRINTX,BOMB,TXPR2,NOPR2	HIWA 400
	COMMON /MMMMMM/ I VPRNT,TV1,TV2,IFIFTH,IFOL,SPACE(10),TSPACE(10)	HIWA 410
	COMMON/NNNNNN/ITEN,ITEM,ITEND2,ND(100,7),I5SEC,VD(100,7)	HIWA 420
	COMMON /UUUUUU/ NSON(100),NSFULL(100),NFT(100),HTEST,VSTARX	HIWA 430
	COMMON /ZZZZZZ/ ENTER,QHOUR,HMIN,HMODE,IR, HBAR2,PCMPLY,PCMP	HIWA 440
C		HIWA 450
C	CHECK VALUES OF 'ENTER', 'NCARS', 'NMILE'	HIWA 460
C		HIWA 470
	CHECK = 0	HIWA 480
	IF(ENTER.EQ.0.OR.ENTER.EQ.1)GO TO 51	HIWA 490
	WRITE(6,50)ENTER	HIWA 500
50	FORMAT(///,1X,'ENTER =',I3,8X,'VALUE OF ENTER IS IN ERROR')	HIWA 510
	CHECK = 1	HIWA 520
51	IF(NCARS.LT.350.AND.NCARS.GT.1)GO TO 53	HIWA 530
	WRITE(6,52)NCARS	HIWA 540
52	FORMAT(///,1X,'NCARS =',I5,8X,'VALUE OF NCARS IS IN ERROR')	HIWA 550
	CHECK = 1	HIWA 560
53	IF(NMILE.GT.0.AND.NMILE.LT.11)GO TO 55	HIWA 570
	WRITE(6,54)NMILE	HIWA 580
54	FORMAT(///,1X,'NMILE =',I5,8X,'VALUE OF NMILE IS IN ERROR')	HIWA 590
	CHECK = 1	HIWA 600
55	IF(CHECK.NE.0)RETURN	HIWA 610
C	IBOMB = NUMBER OF 5 SEC INTERVALS AFTER WHICH PROGRAM WILL	HIWA 620
C	BE ABORTED	HIWA 630
	IBOMB = BOMB*.2 - .01	HIWA 640
C		HIWA 650
C		HIWA 660
C	INITIALIZE VARIABLES	HIWA 670
C		HIWA 680
	IFOL = 0	HIWA 690
	JSEC=0	HIWA 700
	NCARSM = NCARS	HIWA 710
	IF(NCARS .GT. 100) NCARSM = 100	HIWA 720
	VSTARX=VSTART	HIWA 730
	DO 701 I=1,10	HIWA 740

701	SPACE(I) = 0.0	HIWA 750
	TSPACE(I) = 0.0	HIWA 760
C		HIWA 770
C	IFLAG3 = COUNTER FOR PRINTING XCAR, VCAR, ANOW, IMODE	HIWA 780
C		HIWA 790
	IFLAG3 = -1	HIWA 800
	TOTALT = 0.	HIWA 810
	NFEET = 5280 * NMILE	HIWA 820
	FNFEET = FLCAT(NFEET)	HIWA 830
	XFEET = FLCAT(NFEET) * 1.2	HIWA 840
	KSIGN=5.*TAUS + 1.5	HIWA 850
C		HIWA 860
C	HERE TRAVEL(I) IS TIME WHEN I-TH CAR ENTERS HIGHWAY	HIWA 870
C	LATER TRAVEL(I) STORES TRAVEL TIME FOR I-TH CAR	HIWA 880
C		HIWA 890
	TRAVEL(I) = 0.	HIWA 900
	ITEN=10*NMILE	HIWA 910
	ITENP = ITEN + 1	HIWA 920
	ITENM=ITEN-1	HIWA 930
	ITEND2=ITEN/2	HIWA 940
	CYCLEA = 1	HIWA 950
	CARA = 1	HIWA 960
	CYCLE1 = 1	HIWA 970
	CARONE = 1	HIWA 980
	CYCLEZ = 1	HIWA 990
	CARZ = 1	HIWA 1000
	DO 704 I=1,ITEN	HIWA 1010
	NFT(I)=528*I	HIWA 1020
	LSIGN(I)=OFF	HIWA 1030
	NSON(I)=0	HIWA 1040
	NSFULL(I)=0	HIWA 1050
	DO 704 J=1,7	HIWA 1060
	VSIGN(I,J)=9.E10	HIWA 1070
	ND(I,J)=0	HIWA 1080
704	VD(I,J)=9.E10	HIWA 1090
	DO 700 I = 1, NCARS	HIWA 1100
	CECT(I) = 0	HIWA 1110

	ANOW(I) = 0.	HIWA 1120
	IBRNCH(I) = 1	HIWA 1130
	JSTOP(I) = 0.	HIWA 1140
	ATOTAL(I) = 0.	HIWA 1150
700	CONTINUE	HIWA 1160
	IBRNCH(1) = 6	HIWA 1170
	HTEST=3.*HBAR-1.3	HIWA 1180
C		HIWA 1190
C	TIME STARTS WHEN FIRST CAR ENTERS THE HIGHWAY	HIWA 1200
C		HIWA 1210
	TNOW = 0.	HIWA 1220
C		HIWA 1230
C	INITIALIZE POSITION & VELOCITY FOR CAR 1	HIWA 1240
C		HIWA 1250
	DO 703 J = 1, 11	HIWA 1260
	XCAR(12-J)=VSTART * .2*FLAGAT(J-11)	HIWA 127C
703	VCAR(12-J) = VSTART	HIWA 1280
C		HIWA 1290
C	IMAX SPECIFIES NUMBER OF VEHICLES FOR PRINTOUT EVERY NOPRNT CYCLES	HIWA 1300
C		HIWA 1310
	IMAX = 20	HIWA 1320
	IF(NCARS .LT. 20) IMAX = NCARS	HIWA 1330
C		HIWA 1340
C	START OF MAJOR DO-LOOPS	HIWA 1350
C		HIWA 1360
	DO 3000 I30SEC = 1, 120	HIWA 1370
	DO 3000 I30SEC = 1, 2	HIWA 1380
	DO 2000 I5SEC = 2, 7	HIWA 139C
C		HIWA 1400
	TNOW=FLOAT(IFIX(TNOW+.002))+.00002	HIWA 1410
	IF(TNOW .LT. TXPR2 .OR. NOPRNT .EQ. NOPR2) GO TO 1707	HIWA 1420
	IFLAG3= -1	HIWA 1430
	NOPRNT = NOPR2	HIWA 1440
1707	CONTINUE	HIWA 1450
	IVP = 0	HIWA 1460
	JVSET = 0	HIWA 1470
	IF(IVPRNT .EQ. 0) GO TO 707	HIWA 1480

	IF(TNOW .LT. TV1 .OR. TNOW .GT. TV2) GO TO 707	HIWA 1490
	IVP = 1	HIWA 1500
	DO 706 J = 1,5	HIWA 1510
	DO 706 I = 1, NCARSM	HIWA 1520
706	IVSEC(I,J) = 15999	HIWA 1530
707	ISIGN=I5SEC-1	HIWA 1540
	IF(ISIGN.EQ.1) ISIGN=7	HIWA 1550
C		HIWA 1560
C	INITIALIZE ND AND VD FOR CURRENT VALUE OF ISSEC	HIWA 1570
C		HIWA 1580
	DO 410 I=1,ITEN	HIWA 1590
	ND(I,ISSEC)=ND(I,ISIGN)	HIWA 1600
410	VD(I,ISSEC)=VD(I,ISIGN)	HIWA 1610
C		HIWA 1620
	IF(CARONE.NE.CARZ .AND. TNOW.GE.TXPRNT .AND. NOPRNT .LT. 6 .AND.	HIWA 1630
2	CARA .LE. IMAX) WRITE(6,7C5)	HIWA 1640
705	FORMAT(1H1)	HIWA 1650
C		HIWA 1660
	DO 1000 IFIFTH = 1, 25	HIWA 1670
C		HIWA 1680
C		HIWA 1690
	IF(CARA .GE. IMAX) GO TO 995	HIWA 1700
	IFLAG3 = IFLAG3 + 1	HIWA 1710
	IF(IFLAG3.LT.NOPRNT) GO TO 995	HIWA 1720
	IFLAG3 = 0	HIWA 1730
	IF(TNOW .LT. TXPRNT .OR. CARONE .EQ. CARZ) GO TO 995	HIWA 1740
C		HIWA 1750
C	PRINT SPACING, VELOCITY, ACCEL., & IBRANCH EVERY NOPRNT	HIWA 1760
C	CYCLES	HIWA 1770
C		HIWA 1780
C		HIWA 1790
C	SET VELOCITY AND SPACING FOR THOSE CARS NOT ON HIWAY AT TNOW	HIWA 1800
C		HIWA 1810
	DO 991 J=1,CARA	HIWA 1820
	VV(J) = 10.E10	HIWA 1830
	IX(J)=9999999	HIWA 1840
991	IDX(J) = 0	HIWA 1850

IF(CARZ .GT. IMAX) GO TO 993	HIWA 1860
DO 992 J = CARZ, IMAX	HIWA 1870
VV(J) = 10.E10	HIWA 1880
IX(J)=10.E10	HIWA 1890
992 IDX(J) = 0	HIWA 1900
993 CONTINUE	HIWA 1910
C	HIWA 1920
CALL STEPH (TNOW, MIN, ISEC, SEC)	HIWA 1930
C	HIWA 1940
XCARI = XCAR(CYCLEA)	HIWA 1950
J=CARA	HIWA 1960
VV(CARA)=VCAR(CYCLEA)	HIWA 1970
IX(CARA) = XCAR(CYCLEA) +0.5	HIWA 1980
IDX(CARA)=XCAR(CYCLEA) + 0.5	HIWA 1990
I=CYCLEA	HIWA 2000
990 I = I + 11	HIWA 2010
J = J + 1	HIWA 2020
IF(I .GT. 3850) I = I - 3850	HIWA 2030
IDX(J) = XCARI - XCAR(I) + .5	HIWA 2040
IX(J) = XCAR(I) + .5	HIWA 2050
VV(J) = VCAR(I)	HIWA 2060
XCARI = XCAR(I)	HIWA 2070
IF(J .LT. CARZ .AND. J .LT. IMAX) GO TO 99C	HIWA 2080
WRITE(6,692) (IBRNCH(I), I=1, IMAX)	HIWA 2090
WRITE(6,691) (ANOW(I), I=1, IMAX)	HIWA 2100
WRITE(6,690) MIN, ISEC, SEC, (IDX(I), I=1, IMAX)	HIWA 2110
IF(PRINTX .EQ. 1) WRITE(6,692) (IX(I), I=1, IMAX)	HIWA 2120
WRITE(6,691) (VV(I), I=1, IMAX)	HIWA 2130
690 FORMAT(/3H T=, I2, 1+-, I1, F3.1, I8, 19I6)	HIWA 2140
691 FORMAT(12X, 20F6.1)	HIWA 2150
692 FORMAT(12X, 20I6)	HIWA 2160
C	HIWA 2170
C COMPUTE ACCELERATION FOR T = TNOW	HIWA 2180
C COMPUTE VELOCITY AND POSITION FOR T = TNOW + .2	HIWA 2190
C	HIWA 2200
995 IF(IVP .EQ. 0) GO TO 100	HIWA 2210
JVSET=0	HIWA 2220

	XOUT = XCAR(CYCLEA)	HIWA 2970
	CYCLEA = CYCLEA + 11	HIWA 2980
	IF(CYCLEA .GT. 3850) CYCLEA = CYCLEA - 3850	HIWA 2990
	IDXOUT(CARA) = HFIX(XOUT - XCAR(CYCLEA) * 0.5)	HIWA 3000
201	CONTINUE	HIWA 3010
	IF(XCAR(CYCLE1).LT. XFEET) GO TO 202	HIWA 3020
	CARONE = CARONE + 1	HIWA 3030
	CYCLE1 = CYCLE1 + 11	HIWA 3040
	IF(CYCLE1 .GT. 3850) CYCLE1 = CYCLE1 - 3850	HIWA 3050
C		HIWA 3060
202	IF(CARZ .EQ. NCARS) GO TO 205	HIWA 3070
	IF(XCAR(CYCLEZ).LT.HDRV(CARZ+1)*VCAR(CYCLEZ)+20.) GO TO 1000	HIWA 3080
C		HIWA 3090
C	A CAR HAS ENTERED THE HIGHWAY WITHIN THE LAST .2 SECONDS	HIWA 3100
C		HIWA 3110
	DFOL=HDRV(CARZ+1)*VCAR(CYCLEZ)+20.	HIWA 3120
	CARZ = CARZ + 1	HIWA 3130
	TRAVEL(CARZ)=TNOW	HIWA 3140
	CYCLEZ = CYCLEZ + 11	HIWA 3150
	IF (CARZ - CARONE .GE. 350) GO TO 901	HIWA 3160
C	UPDATE ND AND SET CECT FOR ENTERING CAR	HIWA 3170
	CECT(CARZ)=1	HIWA 3180
	ND(1,15SEC)=ND(1,15SEC) +1	HIWA 3190
C		HIWA 3200
	IF (CYCLEZ .GT. 3850) GO TO 204	HIWA 3210
	DO 203 J = 1, 11	HIWA 3220
	I = CYCLEZ + J - 1	HIWA 3230
203	XCAR(I) = XCAR(I-11) -DFOL	HIWA 3240
	VCAR(I) = VCAR(I-11)	HIWA 3250
	VSTARX = VCAR(CYCLEZ)	HIWA 3260
	GO TO 205	HIWA 3270
204	CYCLEZ = CYCLEZ - 3850	HIWA 3280
	DO 206 J = 1, 11	HIWA 3290
	I = CYCLEZ + J - 1	HIWA 3300
	XCAR(I) = XCAR(I+3839) - DFOL	HIWA 3310
206	VCAR(I) = VCAR(I+3839)	HIWA 3320
	VSTARX = VCAR(CYCLEZ)	HIWA 3330

C		HIWA 3340
	205 CONTINUE	HIWA 3350
C		HIWA 3360
	1000 CONTINUE	HIWA 3370
C		HIWA 3380
C	DO THIS EVERY 5 SECONDS	HIWA 3390
C		HIWA 3400
C		HIWA 3410
C	FOR PURPOSES OF EASE IN READING PRINTOUT	HIWA 3420
C	SET VD(I,15SEC) = 9.E10 IF NO CARS ARE ON PREVIOUS SECTIONS	HIWA 3430
C		HIWA 3440
	IF(ND(I,15SEC) .NE. 0) GO TO 321	HIWA 3450
	DO 320 I=2,ITEN	HIWA 3460
	IF(ND(I,15SEC) .NE. 0) GO TO 321	HIWA 3470
320	VD(I-1,15SEC) = 9.E10	HIWA 3480
	VD(ITEN,15SEC) = 9.E10	HIWA 3490
321	CONTINUE	HIWA 3500
C		HIWA 3510
C	CHANGE SIGNS	HIWA 3520
C		HIWA 3530
	IF(CONTRL .EQ. 1) CALL SETSGN	HIWA 3540
C		HIWA 3550
C		HIWA 3560
C	PRINTING OF VELOCITY OF ALL CARS -- IF IVPRT = 1	HIWA 3570
C		HIWA 3580
	IF(IVP .EQ. 0) GO TO 1999	HIWA 3590
	NT5=IFIX(TNOW+.001)	HIWA 3600
	NT4=NT5-1	HIWA 3610
	NT3=NT4-1	HIWA 3620
	NT2=NT3-1	HIWA 3630
	NT1=NT2-1	HIWA 3640
	WRITE(6,250) NT1,NT2,NT3,NT4,NT5,NT1,NT2,NT3,NT4,NT5	HIWA 3650
250	FORMAT(1H1,60X,'VELOCITY' // 37X,'NEGATIVE VALUE MEANS CAR IS RESP	HIWA 3660
	ONDING TO A SIGN' // 9X, 5('T=',13,5X), 15X, 5('T=',13,5X) //)	HIWA 3670
	LZ=50	HIWA 3680
	IF(CARZ.LT.50)LZ=CARZ	HIWA 3690
	DO 253 I=1,LZ	HIWA 3700

	WRITE(6,251) I,(IVSEC(I,J),J=1,5)	HIWA 3710
251	FORMAT(1X,I3,6X,5(I4,6X))	HIWA 3720
	IPL=I+50	HIWA 3730
	IF(IPL.GT. CARZ) GO TO 253	HIWA 3740
	WRITE(6,252) IPL,(IVSEC(IPL,J),J=1,5)	HIWA 3750
252	FORMAT(1H+,67X,I3,4X,5(I4,6X))	HIWA 3760
253	CONTINUE	HIWA 3770
	JSEC=0	HIWA 3780
C		HIWA 3790
C		HIWA 3800
	1999 IBOMB = IBOMB - 1	HIWA 3810
	IF(IBOMB .LT. 0) RETURN	HIWA 3820
	2000 CONTINUE	HIWA 3830
C		HIWA 3840
C		HIWA 3850
C	DO THIS EVERY 30 SECONDS	HIWA 3860
C		HIWA 3870
C	WRITE ND & VD & VSIGN	HIWA 3880
C		HIWA 3890
	CALL STEPH(TNOW,MIN,I SEC, SEC)	HIWA 3900
	WRITE(6,680) MIN, I SEC, SEC	HIWA 3910
680	FORMAT(1H1,10X,'*** TIME =',I3,' MIN.',I2,F3.1,' SEC.'//5H MILE,	HIWA 3920
	1 6(' ND VD VSIGN '), ' FEET'/)	HIWA 3930
	IBB = XCAR(CYCLEZ)/528.0 + 1.0	HIWA 3940
	IBB=IBB-7	HIWA 3950
	IF(IBB.LT.1) IBB=1	HIWA 3960
	IAA = XCAR(CYCLEA)/528.0 + 1.0	HIWA 3970
	DO 681 I = IBB, IAA	HIWA 3980
	X = FLCAT(I)/10.	HIWA 3990
	IIX = I * 528	HIWA 4000
	ZD(2)=VD(I,2)*.6818182+.0001	HIWA 4010
	ZD(3)=VD(I,3)*.6818182+.0001	HIWA 4020
	ZD(4)=VD(I,4)*.6818182+.0001	HIWA 4030
	ZD(5)=VD(I,5)*.6818182+.0001	HIWA 4040
	ZD(6)=VD(I,6)*.6818182+.0001	HIWA 4050
	ZD(7)=VD(I,7)*.6818182+.0001	HIWA 4060
	ZSIGN(2)=VSIGN(I,2)*.683	HIWA 4070

ZSIGN(3)=VSIGN(I,3)*.683	HIWA 4080
ZSIGN(4)=VSIGN(I,4)*.683	HIWA 4090
ZSIGN(5)=VSIGN(I,5)*.683	HIWA 4100
ZSIGN(6)=VSIGN(I,6)*.683	HIWA 4110
ZSIGN(7)=VSIGN(I,7)*.683	HIWA 4120
WRITE(6,684) X, (ND(I,J),ZD(J),ZSIGN(J),J=2,7), IIX	HIWA 4130
684 FORMAT(1X,F4.1,6(14,2X,14,3X,13,4X),15)	HIWA 4140
VSIGN(I,1)=VSIGN(I,7)	HIWA 4150
681 CONTINUE	HIWA 4160
DO 219 I = 1, ITEN	HIWA 4170
ND(I,1) = ND(I,7)	HIWA 4180
219 VD(I,1) = VD(I,7)	HIWA 4190
C	HIWA 4200
3000 CONTINUE	HIWA 4210
WRITE(6,603)	HIWA 4220
603 FORMAT(//18H ** ONE HOUR IS UP)	HIWA 4230
RETURN	HIWA 4240
C	HIWA 4250
C ALL CARS HAVE LEFT THE HIGHWAY	HIWA 4260
C	HIWA 4270
C	HIWA 4280
1300 CALL FINISH	HIWA 4290
C	HIWA 4300
RETURN	HIWA 4310
901 WRITE(6,604)	HIWA 4320
604 FORMAT(//20H ** 350 CARS ON ROAD)	HIWA 4330
RETURN	HIWA 4340
END	HIWA 4350

	SUBROUTINE TITLE	TITL	10
	IMPLICIT INTEGER(C,I-N)	TITL	20
	INTEGER*2 IBRNCH, IDMODE, JSEC, JVSET, IVSEC(100,5), CECT, COMPLY	TITL	30
	INTEGER ON, OFF, ENTER	TITL	40
	COMMON /AAAAA/ NCARS, NMILE, VSTART, TXPRNT, IBRNCH(350),	TITL	50
	1 CARNOW, NOPRNT, CONTRL, ON, OFF, BETA, TAUS, KSIGN, LSIGN(100),	TITL	60
	2 HBAR, HDRV(350), COMPLY(350), VSIGN(100,7)	TITL	70
	COMMON /BBBBBB/ TF3(5), AF3(4), BASE, NREP, NREPI, VI3	TITL	80
	COMMON /CCCCC/ CBUG1, CROUND, SGNC	TITL	90
	COMMON /GGGGG/ ISIGN, JSEC, JVSET, IVSEC, IVP, ITENP, CECT(350)	TITL	100
	COMMON /TTTTT/ IDMODE(350), TAUA, TAUD, VLIMIT, KA, KD	TITL	110
	COMMON /ZZZZZ/ ENTER, QHOUR, HMIN, HMODE, IR, HBAR2, PCMPLY, PCMP	TITL	120
	COMMON /KKKKK/ ESIG2, ETSTOP, ETOTLT, EPER, EPCMP, IEVAV1, IEDX, NENS,	TITL	130
	2 JENS, EQ, ESPACE(10), RRI	TITL	140
	Y = .5280./3600.	TITL	150
	KA = 5.*TAUA + 1.5	TITL	160
	KD = 5.*TAUD + 1.5	TITL	170
	WRITE(6,601) NMILE, NCARS	TITL	180
601	FORMAT(1H1,20X,'*** HIGHWAY SIMULATION PROGRAM ***'//4X,'INPUT DAT	TITL	190
	1A FOR THIS RUN'/10X,'NMILE =',I4,' = LENGTH OF HIGHWAY IN MILES'/	TITL	200
	2 10X,'NCARS =',I4,' = TOTAL NUMBER OF CARS TO ENTER THE HIGHWAY')	TITL	210
	VSTART = VI3*Y	TITL	220
	VLIMIT=VSTART+14.667	TITL	230
	BASE = 0.0	TITL	240
	NREPI=0	TITL	250
C		TITL	260
C	PRINT HEADING FOR F3	TITL	270
C		TITL	280
	WRITE(6,612) TF3,AF3,NREP,VI3	TITL	290
612	FORMAT(1H0,5X,'THIS RUN USES SUBROUTINE F3'////	TITL	300
	2 6X,'TF3(I) =',5F12.2///	TITL	310
	3 6X,'AF3(I) =',4F12.2///	TITL	320
	4 6X,'NREP =',I5///	TITL	330
	5 6X,'INITIAL VELOCITY = VI3 =',F8.1,' MI/HR'////)	TITL	340
	TF3(1) = TF3(1) - 0.02*RR1	TITL	350
	TF3(2) = TF3(2) - 0.02*RR1	TITL	360
	TF3(3) = TF3(3) - 0.02*RR1	TITL	370

	TF3(4) = TF3(4) - 0.02*RR1	TITL 380
	TF3(5) = TF3(5) - 0.02*RR1	TITL 390
	RR1 = 0.0	TITL 400
C		TITL 410
	QSEC = QHOUR/3600.	TITL 420
	HBAR = 1./QSEC - 20./VSTART	TITL 430
	HDRV(1) = 0.	TITL 440
	IDMODE(1) = 0	TITL 450
	DO 201 I = 2, NCARS	TITL 460
	IDMODE(I) = 0	TITL 470
	HDRV(I) = HBAR	TITL 480
201	CONTINUE	TITL 490
	IF(ENTER.EQ.0) GO TO 999	TITL 500
	IF(ENTER .GT. 1) GO TO 999	TITL 510
C		TITL 520
C	HEADWAY FACTORS COMPUTED	TITL 530
C		TITL 540
	TBAR=1./QSEC	TITL 550
	HEIGHT=2./(3.*HBAR-2.*HMIN-HMODE)	TITL 560
	HMAX=3.*HBAR-HMIN-HMODE	TITL 570
	HRVSUM=0.	TITL 580
C	SET IS=IR --- SAME STARTING VALUE FOR ALL RUNS	TITL 590
	IS=IR	TITL 600
	DO 104 I = 2, NCARS	TITL 610
	CALL RANDU(IS, IV, Z)	TITL 620
	IS = IV	TITL 630
	IF(Z.GT.((HMODE-HMIN)/(HMAX-HMIN))) GO TO 103	TITL 640
	HDRV(I)=SQRT((HMAX-HMIN)*(HMODE-HMIN)*Z)+HMIN	TITL 650
	GO TO 104	TITL 660
103	HDRV(I) = -SQRT((HMAX-HMODE)*(HMAX-HMIN)*(1.-Z))+HMAX	TITL 670
104	HRVSUM=HRVSUM+HDRV(I)	TITL 680
	HBAR2=HRVSUM/FLOAT(NCARS-1)	TITL 690
	WRITE(6,620) QHOUR, TBAR, HBAR, HBAR2, HMIN, HMODE, HMAX, HEIGHT	TITL 700
620	FORMAT('0THIS RUN IS MADE WITH RANDOMLY GENERATED HEADWAYS'///	TITL 710
	2 5X, 'QHOUR = AVERAGE FLOW =', F5.0, ' VEHICLES/HOUR'//	TITL 720
	3 5X, 'TBAR = 1/QSEC = AVG. TIME HEADWAY =', F5.2, ' SECONDS'//	TITL 730
	4 5X, 'HBAR = AVERAGE HEADWAY FACTOR DESIRED =', F6.3//	TITL 740

5	5X,'HBAR2 = AVERAGE HEADWAY FACTOR GENERATED ='	F6.3////	TITL	750
6	' FOR THE DENSITY FUNCTION USED'//		TITL	760
7	5X,'HMIN = MINIMUM HEADWAY FACTOR ='	F5.2,' (GIVEN)'/	TITL	770
8	5X,'HMODE = MODAL HEADWAY FACTOR ='	F5.2,' (GIVEN)'/	TITL	780
9	5X,'HMAX = MAXIMUM HEADWAY FACTOR ='	F5.2,' (CALCULATED)'/	TITL	790
A	5X,'HEIGHT = MAX OF DENSITY FUNCTION ='	F6.3,' (CALCULATED)'/	TITL	800
	WRITE(6,630)		TITL	810
630	FORMAT(////24X,'F(H)'/		TITL	820
2	25X,'+'//		TITL	830
3	19X,'HEIGHT-----*'/		TITL	840
4	25X,'+',10X,'* ' *'/		TITL	850
5	25X,'+',8X,'* ' *'/		TITL	860
6	25X,'+',6X,'* ' *',8X,'*'/		TITL	870
7	25X,'+++++*****+*****+*****+*****+***** H'/		TITL	880
8	28X,'HMIN',4X,'HMODE',8X,'HMAX'//1H10		TITL	890
999	IF(CONTRL .EQ. 0) GO TO 9999		TITL	900
C			TITL	910
C	COMPUTE SIGN COMPLIANCE FACTOR FOR EACH DRIVER		TITL	920
C			TITL	930
	CMP=0		TITL	940
	IS=2*IR-37		TITL	950
	DO 105 I=2,NCARS		TITL	960
	CALL RANDU(IS,IY,Z)		TITL	970
	IS=IY		TITL	980
	COMPLY(I)=0		TITL	990
	IF(Z .LT. PCMPLY) COMPLY(I)=1		TITL	1000
105	CMP=CMP+COMPLY(I)		TITL	1010
	PCMP=FLOAT(CMP)/FLOAT(NCARS-1)		TITL	1020
9999	RETURN		TITL	1030
	END		TITL	1040

C	IDMODE = 1 MAX DECEL. NOT LIMITED SINCE SPACING IS	G1	380
C	UNSAFE	G1	390
C		G1	400
C	AAA = NEW VALUE OF ACCEL. FOR (CARNOW-1)-TH CAR	G1	410
C		G1	420
C	COMPUTE ACTUAL & DESIRED SPACINGS	G1	430
C		G1	440
	H = HDRV(CARNOW)	G1	450
	DELTA V = VCAR(CARFOL+1) - VCAR(CARNO+1)	G1	460
	SA = XCAR(CARNO+1) - XCAR(CARFOL+1) - 20.	G1	470
	SD = H*VCAR(CARNO+1)	G1	480
	DA = SA + 20.	G1	490
	DD = SD + 20.	G1	500
	SAOVSD = 1.0	G1	510
	IF(SD .GT. 5.0) SAOVSD = SA/SD	G1	520
	AAA = ANOW(CARNOW-1)	G1	530
C		G1	540
C	SEE WHICH MODE THIS CAR WAS IN (LAST TIME)	G1	550
C		G1	560
	IF(IBRNCH(CARNOW) .GT. 50) IBRNCH(CARNOW) = IBRNCH(CARNOW) - 50	G1	570
	IBR = IBRNCH(CARNOW)	G1	580
	ASSIGN 300 TO IBACK	G1	590
	IF(IBR .GE. 10) GO TO 7000	G1	600
C		G1	610
C	LAST TIME CAR WAS IN DISTANCE DETECTION MODE (MODE ZERO)	G1	620
C	CHECK TO SEE IF CAR REMAINS IN DISTANCE DETECTION MODE	G1	630
C		G1	640
	IF(ABS(AAA) .LE. 1.) GC TC (1,2,3,4,5),IBR	G1	650
	IF(IBR .NE. 1) GO TO 250	G1	660
	IBRNCH(CARNOW) = 10	G1	670
	GO TC 7001	G1	680
250	IF(AAA .LT. -1.) GO TO 252	G1	690
C		G1	700
C	CAR AHEAD HAS ACCELERATION .GT. +1.0	G1	710
C		G1	720
	IF(IBR .EQ. 2) GO TO 2	G1	730
	IF(IBR .EQ. 5) GC TC 5	G1	740

	ASSIGN 254 TO IBACK	G1	750
	GO TO 7001	G1	760
254	IF(ACF .GT. 2.0) GO TO 258	G1	770
	IF(IBR .EQ. 4) GO TO 4	G1	780
	GO TO 3	G1	790
C		G1	800
C	CAR AHEAD HAS ACCELERATION .LT. -1.0	G1	810
C		G1	820
252	IF(IBR .EQ. 3) GO TO 3	G1	830
	IF(IBR .EQ. 4) GO TO 4	G1	840
	ASSIGN 256 TO IBACK	G1	850
	GO TO 7001	G1	860
256	IF(ACF .LT. ANOW(CARNOW)) GO TO 258	G1	870
	IF(IBR .EQ. 2) GO TO 2	G1	880
	GO TO 5	G1	890
258	IBRNCH(CARNOW) = 10	G1	900
	GO TO 300	G1	910
C		G1	920
C		G1	930
C	+++++	G1	940
C	+ ENTER DISTANCE DETECTION MODE +	G1	950
C	+++++	G1	960
1	CONTINUE	G1	970
	Q = DA/DD	G1	980
	IF(Q .LT. 0.85 + 1./(VCAR(CARNO+1)+10.)) GO TO 2	G1	990
	IF(Q .GT. 1.2) GO TO 4	G1	1000
C		G1	1010
C	ACTUAL SPACING WITHIN 0.1 OF DESIRED SPACING	G1	1020
C		G1	1030
	ANOW(CARNOW)=0.	G1	1040
	IBRNCH(CARNOW) = 1	G1	1050
	GO TO 990	G1	1060
C		G1	1070
C	FOLLOWING TOO CLOSE	G1	1080
C		G1	1090
2	CONTINUE	G1	1100
	IF(DELTA V .GT. 0.) GO TO 22	G1	1110

C		G1	1120
C	TOO CLOSE BUT SEPARATION IS INCREASING	G1	1130
C		G1	1140
	IF(DD-DA .GT. .25*DELTAV*DELTAV) GO TO 22	G1	1150
C		G1	1160
C	TOO CLOSE -- V2 .LT. V1 -- ACCELERATE UNTIL V2 = V1	G1	1170
C		G1	1180
21	ANOW(CARNOW) = 2.	G1	1190
	IBRNCH(CARNOW) = 3	G1	1200
	GO TO 990	G1	1210
C		G1	1220
C	TOO CLOSE -- DECELERATE BY ENGINE BRAKING - FOOT OFF GAS	G1	1230
C	USE 80 PER-CENT OF FULL ENGINE BRAKING	G1	1240
C		G1	1250
22	ANOW(CARNOW) = (-0.03114286*VCAR(CARFOL+1) - 0.38955)*0.80	G1	1260
	IF(VCAR(CARFOL+1) .LT. 27.629) ANOW(CARNOW) = -1.0	G1	1270
	IBRNCH(CARNOW) = 2	G1	1280
	GO TO 990	G1	1290
3	CONTINUE	G1	1300
	IF(DELTAV .LT. -.2) GO TO 21	G1	1310
	ANOW(CARNOW) = 0.	G1	1320
	IBRNCH(CARNOW) = 1	G1	1330
	GO TO 990	G1	1340
C		G1	1350
C	FOLLOWING TOO FAR	G1	1360
C		G1	1370
4	CONTINUE	G1	1380
	IF(DELTAV .LE. 0.) GO TO 42	G1	1390
C		G1	1400
C	COMPUTE CHANGE IN SPACING THAT WOULD RESULT	G1	1410
C	IF DRIVER STOPPED ACCELERATING AT TNOW	G1	1420
C		G1	1430
	SAMSD = SA - SD	G1	1440
	IF(VCAR(CARFOL+1) .LE. 27.629) GO TO 46	G1	1450
	IF(VCAR(CARNOW+1) .LT. 27.629) GO TO 45	G1	1460
	BPAV1 = (0.03114286*VCAR(CARNOW+1) + 0.38955)*0.80	G1	1470
	ARG = (0.03114286 * VCAR(CARFOL+1) + 0.38955) / BPAV1*0.80	G1	1480

	DS2=(DEL TAV-BPAV1*ALOG(ARG)/.03114286/.8)/.03114286/.8	G1	1490
	GO TO 47	G1	1500
45	BPAV1 = (.03114286*27.629 + 0.38955)*0.80	G1	1510
	ARG = (0.03114286 * VCAR(CARFOL+1) + 0.38955) / BPAV1*0.80	G1	1520
	DS2 = (VCAR(CARFOL+1) - 27.629 -BPAV1 * ALOG(ARG) / 0.80/	G1	1530
1	0.03114286)/.03114286/.8 + (27.629 -VCAR(CARNO+1))*(27.629-	G1	1540
2	VCAR(CARNO+1))*0.5	G1	1550
	GO TO 47	G1	1560
46	DS2=-DEL TAV*.2 - .2*.2*.5	G1	1570
C		G1	1580
C	SHOULD DRIVER REMOVE FOOT FROM GAS NOW?	G1	1590
C		G1	1600
47	IF(SAMSD .GT. DS2) GO TO 42	G1	1610
C		G1	1620
C	USE 80 PER-CENT OF FULL ENGINE BRAKING	G1	1630
C		G1	1640
41	ANOW(CARNOW) = (-0.03114286*VCAR(CARFOL+1) - 0.38955)*0.80	G1	1650
	IF(VCAR(CARFOL+1) .LT. 27.629) ANOW(CARNOW) =-1.0	G1	1660
	IBRNCH(CARNOW) = 5	G1	1670
	GO TO 990	G1	1680
42	ANOW(CARNOW) = 2.	G1	1690
	IF(DELTAV .GT.12.0) ANOW(CARNOW)=0.	G1	1700
	IBRNCH(CARNOW) = 4	G1	1710
	GO TO 990	G1	1720
C	FOOT OFF GAS -- V2 .GT. V1 -- STAY IN MODE 5 UNTIL V2=V1	G1	1730
C	(APPROX.)	G1	1740
5	CONTINUE	G1	1750
	IF(DEL TAV .GT. -.1*ANOW(CARNOW)) GO TO 41	G1	1760
	ANOW(CARNOW) = 0.	G1	1770
	IBRNCH(CARNOW) = 1	G1	1780
	GO TO 990	G1	1790
C	-----	G1	1800
C	- END OF DD MODE -	G1	1810
C	-----	G1	1820
C		G1	1830
C	LAST TIME CAR WAS IN CAR-FOLLOWING MODE	G1	1840
C		G1	1850

7000	IF (ABS(AAA) .GT. 1.) IBRNCH(CARNOW) = 9	G1	1860
	IBRNCH(CARNOW) = IBRNCH(CARNOW) + 1	G1	1870
	IF (IBRNCH(CARNOW) .LT. 17) GO TO 7001	G1	1880
C		G1	1890
C	CHECK IF ANOW IS O.K. FOR CARNOW-TH CAR TO ENTER DD MODE	G1	1900
C		G1	1910
	IF (ANOW(CARNOW) .LT. (-.03114286 *VCAR(CARFOL+1) -.38955) *.8 .OR.	G1	1920
1	ANOW(CARNOW) .GT. 2.) GO TO 7002	G1	1930
	IF (ABS(DELTA V)/(SA+1.)/(SA+1.) .GT. 2.E-4) GO TO 7002	G1	1940
	IBRNCH(CARNOW) = 1	G1	1950
	GO TO 1	G1	1960
C	+++++	G1	1970
C	+ ENTER CAR FOLLOWING MODE (MCDE ONE) +	G1	1980
C	+++++	G1	1990
7002	IBRNCH(CARNOW) = IBRNCH(CARNOW) - 1	G1	2000
7001	DELVD = VCAR(CARNO+KD) - VCAR(CARFOL+KD)	G1	2010
	DELVA = VCAR(CARNO+KA) - VCAR(CARFOL+KA)	G1	2020
	IF (DELVD .LT. 0.) GO TO 102	G1	2030
	IF (DELVA .LT. 0.) GO TO 101	G1	2040
C		G1	2050
C	CAR EXPECTING TO ACCELERATE	G1	2060
C		G1	2070
	AMAX = 12.4 - 0.0913*VCAR(CARFOL+1)	G1	2080
	ACF = AMAX	G1	2090
	IF (DELVA .LT. 2.*AMAX*H) ACF = DELVA/H*(1.-.25/H*DELVA/AMAX)	G1	2100
	IF (ACF .GT. 2.0) GO TO 93	G1	2110
C		G1	2120
C	IN C.F. MODE AND ANOW(CARNOW) .GE. 0. .AND. .LT. 2.0	G1	2130
C	CHECK IF ACCEL. SHOULD BE INCREASED DUE	G1	2140
C	TO LARGE FOLLOWING DISTANCE.	G1	2150
C	ADD TERM TO ACCEL. IF SA/SD .GT. 1.2	G1	2160
C		G1	2170
	IF (SAOVSD .LT. 1.2) GO TO 94	G1	2180
	FA = 2.0	G1	2190
	IF (SAOVSD .LT. 1.4) FA = (SAOVSD-1.2)*10.	G1	2200
	IF (ACF .GT. FA) GO TO 93	G1	2210
	ACF = FA	G1	2220

C		G1	2230
C	CORRECT ACCEL. IF SA/SD .LT. 1.6 .AND. ANOW(CARNOW-1) .LT. 0.	G1	2240
C		G1	2250
C		G1	2260
93	IF(SAOVSD .GT. 1.6) GO TO 1000	G1	2270
94	IF(DELTAV .LT. 0. .OR. AAA .GT. 0.) GO TO 1000	G1	2280
	IF(AAA .LT. -4.0) GO TO 101	G1	2290
	ACF = (1.0 + AAA/4.0)*ACF	G1	2300
	GO TO 1000	G1	2310
101	ACF = 0.	G1	2320
	GO TO 1000	G1	2330
C		G1	2340
C	CAR EXPECTING TO DECELERATE	G1	2350
C		G1	2360
102	FACTOR = 1.0	G1	2370
	IF(VCAR(CARNO+1) .LE. 20.0) GO TO 107	G1	2380
	ADV = 20.0 / VCAR(CARNO+1)	G1	2390
	FACTOR = ADV + (1.-ADV)/SAOVSD	G1	2400
107	PDELVD = FACTOR * DELVD/H	G1	2410
	IF(AAA .GE. 0. .OR. SAOVSD .GT. 1.6) GO TO 108	G1	2420
	PDELVD = PDELVD + SQRT((1.6-SAOVSD)/9.6)*AAA	G1	2430
108	IF(PDELVD .GE. -10.) GO TO 103	G1	2440
C	SSAFE = 1/(2*AMAX)*(V1**2 - V2**2 + (AMAX-A2)*	G1	2450
C	(2*V2*TAU+A2*TAU**2)	G1	2460
C	HERE TAU = 0.45 SEC.	G1	2470
	IF(IDMODE(CARNOW) .EQ. 1) GO TO 104	G1	2480
	SSAFE = 1./48.*(VCAR(CARFOL+1)**2 - VCAR(CARNO+1)**2 + (24.*ANOW(G1	2490
	ICARNOW))*(.9*VCAR(CARFOL+1)+0.2025*ANOW(CARNOW)))	G1	2500
	IF(SA.LT.SSAFE) GO TO 105	G1	2510
	ACF = -10.	G1	2520
	GO TO 106	G1	2530
105	IDMODE(CARNOW) = 1	G1	2540
	GO TO 104	G1	2550
103	IDMODE(CARNOW) = 0	G1	2560
104	ACF = PDELVD	G1	2570
106	IF(DELTAV .GT. 0. .OR. AAA .LT. 0.) GO TO 1000	G1	2580
	IF(AAA .GT. 4.0) GO TO 101	G1	2590

	ACF = (1.0 - AAA/4.0) * ACF	G1	2600
C	-----	G1	2610
C	- END OF CF MODE -	G1	2620
C	-----	G1	2630
	1000 CONTINUE	G1	2640
C **		G1	2650
C **		G1	2660
	GO TO IBACK, (254,256,300)	G1	2670
C **		G1	2680
C **		G1	2690
300	ANOW(CARNOW) = ACF	G1	2700
C	*****	G1	2710
C	* COMPUTE ACCELERATION DUE TO SIGNS *	G1	2720
C	*****	G1	2730
990	IF(CONTRL.EQ.0 .OR. COMPLY(CARNOW).EQ.0) GO TO 992	G1	2740
C	LOCATE SIGN THAT DRIVER SAW TAUS SECONDS AGO	G1	2750
	NSIGN = XCAR(CARFOL + KSIGN)/528. + 1.	G1	2760
	IF(NSIGN .LT. 1) GO TO 992	G1	2770
	IF(NSIGN .GT. ITEN) GO TO 992	G1	2780
C	WAS VSIGN CHANGED WITHIN LAST TAUS SECONDS	G1	2790
	IF(IFIFTH .LT. KSIGN) GO TO 980	G1	2800
C	VSIGN WAS NOT CHANGED WITHIN LAST TAUS SECONDS	G1	2810
	IF(LSIGN(NSIGN) .EQ. OFF) GO TO 992	G1	2820
	MSIGN = ISIGN	G1	2830
	GO TO 985	G1	2840
C	VSIGN WAS CHANGED WITHIN LAST TAUS SECONDS	G1	2850
980	MSIGN = ISIGN - 1	G1	2860
	IF(MSIGN .EQ. 1) MSIGN = 7	G1	2870
C	WAS NSIGN ON DURING PREVIOUS 5 SEC INTERVAL	G1	2880
	IF(VSIGN(NSIGN,MSIGN) .GT. 1.E8) GO TO 992	G1	2890
C		G1	2900
C	ASIGN = ACCELERATION DUE TO SIGNS	G1	2910
C		G1	2920
985	ASIGN = BETA*(VSIGN(NSIGN,MSIGN)-VCAR(CARFOL+KSIGN))	G1	2930
	IF(ASIGN .GE. ANOW(CARNOW)) GO TO 992	G1	2940
	IFCL=1	G1	2950
	ANOW(CARNOW)=ASIGN	G1	2960

	IBRNCH(CARNOW) = IBRNCH(CARNOW) + 50	G1	2970
C		G1	2980
C	*****	G1	2990
C	UPDATE SECTION -- ANOW(CARNOW) HAS JUST BEEN COMPUTED	G1	3000
C	*****	G1	3010
C		G1	3020
	992 VCAR(CARFOL) = VCAR(CARFOL+1) + .2*ANOW(CARNOW)	G1	3030
	IF(VCAR(CARFOL).LT.VLIMIT) GO TO 510	G1	3040
	ANOW(CARNOW)=(VLIMIT-VCAR(CARFOL+1))*5.	G1	3050
	VCAR(CARFOL)=VLIMIT	G1	3060
	GO TO 991	G1	3070
	510 IF(VCAR(CARFOL) .GE. 0.) GO TO 991	G1	3080
	VCAR(CARFOL) = 0.	G1	3090
	ANOW(CARNOW) = -5.*VCAR(CARFOL+1)	G1	3100
	991 XCAR(CARFOL) =XCAR(CARFOL+1)+VCAR(CARFOL+1)*.2 + .02*ANOW(CARNOW)	G1	3110
	IF(XCAR(CARFOL) - XCAR(CARFOL) .GE. 20.) GO TO 200	G1	3120
	XCAR(CARFOL) = XCAR(CARFOL) - 20.	G1	3130
	VCAR(CARFOL) = 5.*(XCAR(CARFOL)-XCAR(CARFOL+1))	G1	3140
	ANOW(CARNOW) = 5.*(VCAR(CARFOL) - VCAR(CARFOL+1))	G1	3150
C		G1	3160
C	UPDATE ND AND VD -- DETECTOR INFORMATION	G1	3170
C		G1	3180
	200 C1=CECT(CARNOW)	G1	3190
	C2=XCAR(CARFOL)/528. + 1.	G1	3200
	IF(C2 .EQ. C1) GO TO 601	G1	3210
	CECT(CARNOW)=C2	G1	3220
	IF(C2 .GT. ITENP) GO TO 601	G1	3230
	ND(C1,15SEC)=ND(C1,15SEC) - 1	G1	3240
	VD(C1,15SEC)=VCAR(CARFOL)	G1	3250
	IF(C2 .EQ. ITENP) GO TO 601	G1	3260
	600 ND(C2,15SEC)=ND(C2,15SEC) + 1	G1	3270
C		G1	3280
C		G1	3290
	601 IF(MOD(CARNOW,10).NE.0.OR.CARNOW.LT.CARA) GO TO 602	G1	3300
	JSP = CARNOW/10	G1	3310
	SPACE(JSP) = SPACE(JSP) + XCAR(CARNC) - XCAR(CARFOL)	G1	3320
	TSPACE(JSP) = TSPACE(JSP) + 1.0	G1	3330

```
602 IF(JVSET .EQ. 0 .OR. CARNOW .GT. 100) RETURN
    IVSEC(CARNOW,JSEC) = MFIX(VCAR(CARFCL) * .5)
    IF(IFCL.EQ.1) IVSEC(CARNOW,JSEC)=-IVSEC(CARNOW,JSEC)
    IFCL = 0
    RETURN
    END
```

```
G1 3340
G1 3350
G1 3360
G1 3370
G1 3380
G1 3390
```

	SUBROUTINE ZAP	ZAP	10
	INTEGER ENTER	ZAP	20
	COMMON /KKKKK/ESIG2,ETSTOP,ETOTLT,EPER,EPCMP,IEVAV1,IEDX,NENS,	ZAP	30
	2 JENS,EQ,ESPACE(10),RR1	ZAP	40
	COMMON /MMMMM/IVPRNT,TV1,TV2,IFIFTH,IFQL,SPACE(10),TSPACE(10)	ZAP	50
	COMMON /ZZZZZ/ENTER,QHOUR,HMIN,HMODE,IR, HBAR2,PCMPY,PCMP	ZAP	60
	DIMENSION IRENS(10)	ZAP	70
	DATA IRENS/692739,688379,362991,443107,967765,619915,727273,	ZAP	80
	2 866531,209831,747299/	ZAP	90
	RR1 = 1.0	ZAP	100
	ESIG2=0.	ZAP	110
	ETSTOP=0.	ZAP	120
	ETOTLT=0.	ZAP	130
	EQ=0.	ZAP	140
	EPER=0.	ZAP	150
	EPCMP=0.	ZAP	160
	IEVAV1=0	ZAP	170
	IEDX=0	ZAP	180
	IF(IVPRNT .EQ. 1) IVSAVE = 1	ZAP	190
	DO 50 I= 1,10	ZAP	200
50	ESPACE(I) = 0.0	ZAP	210
C		ZAP	220
	DO 100 JENS=1,NENS	ZAP	230
	IR=IRENS(JENS)	ZAP	240
	CALL TITLE	ZAP	250
	CALL HIWAY	ZAP	260
	IVPRNT = 0	ZAP	270
100	CONTINUE	ZAP	280
	IF(IVSAVE .EQ. 1) IVPRNT = 1	ZAP	290
	RETURN	ZAP	300
	END	ZAP	310

	SUBROUTINE SETSGN	SETS	10
	IMPLICIT INTEGER(C,I-N)	SETS	20
	INTEGER*2 ND,IBRNCH,NSON,NSFULL,LMTAG(100),COMPLY	SETS	30
	INTEGER ON, OFF	SETS	40
	DIMENSION HDKK(100)	SETS	50
	COMMON XCAR(3861),VCAR(3861),ANOW(350),CYCLE1,CARONE,TNOW,CARA,	SETS	60
	1 CARZ, CYCLEZ, CYCLEA	SETS	70
	COMMON /AAAAAA/ NCARS,NMILE,VSTART,TXPRNT, IBRNCH(350),	SETS	80
	1 CARNOW, NOPRNT,CONTRL,ON,OFF,BETA,TAUS,KSIGN,LSIGN(100),	SETS	90
	2 HBAR,HDRV(350),COMPLY(350),VSIGN(100,7)	SETS	100
	COMMON /CCCCC/ CBUG1,CROUND,SGNC	SETS	110
	COMMON/NNNNN/ITEN,ITENM,ITEND2,ND(100,7),I5SEC,VD(100,7)	SETS	120
	COMMON /UUUUU/ NSON(100),NSFULL(100),NFT(100),HTEST,VSTARX	SETS	130
	DATA Y/7.333333/	SETS	140
	IF(CBUG1 .EQ. 0) GO TO 201	SETS	150
	CALL STEPH(TNOW,MIN,I5SEC,SEC)	SETS	160
	WRITE(6,900)MIN,I5SEC,SEC	SETS	170
900	FORMAT(1H1,'T=',I2,'-',I1,F3.1,////)	SETS	180
	DO 200 K=1,NMILE	SETS	190
	K2 = 10*K	SETS	200
	K1 = K2 - 9	SETS	210
	IF(K .EQ. 6) WRITE(6,914)	SETS	220
914	FORMAT(1H1)	SETS	230
200	WRITE(6,901)((I,I=K1,K2),(I,I=K1,K2),(ND(I,I5SEC),I=K1,K2),	SETS	240
	1(VD(I,I5SEC),I=K1,K2),(NFT(I),I=K1,K2)	SETS	250
901	FORMAT(3X,10(10X,'S',I2)/1X,'*',10(12X,'*')/1X,'*',10(1X,'SECTION	SETS	260
	1',I2,1X,'*')/1X,'*',10(3X,'ND=',I2,4X,'*')/3X,10(8X,F5.1)/	SETS	270
	2 3X,10(8X,I5)////)	SETS	280
201	CONTINUE	SETS	290
C		SETS	300
C	LOCATE SECTION OCCUPIED BY FIRST CAR AND BY LAST CAR	SETS	310
C	LAST CAR ON SECTION NL	SETS	320
C	FIRST CAR ON SECTION NF	SETS	330
C		SETS	340
	NL=XCAR(CYCLEZ)/528.+1.	SETS	350
	NF=XCAR(CYCLEA)/528.+1.	SETS	360
	IF(CBUG1.EQ. 1) WRITE(6,910)NF,NL	SETS	370

910	FORMAT(1X, 'FIRST CAR IS ON SECTION', I3, ' LAST CAR IS ON SECTION'	SETS	380
	1, I3)	SETS	390
C		SETS	400
C	SET ALL SIGNS OFF	SETS	410
C		SETS	420
	DO 300 I=1, ITEN	SETS	430
	LMTAG(I) = 9999	SETS	440
	VSIGN(I, I5SEC)=1.E10	SETS	450
300	LSIGN(I)=OFF	SETS	460
C		SETS	470
C	CHECK IF SIGN 1 SHOULD BE SET	SETS	480
C	ARE THERE ANY CARS ON SECTION 1	SETS	490
C		SETS	500
	IF(CARZ.EQ.NCARS.OR.NL.NE.1.OR.NF.EQ.1) GO TO 320	SETS	510
C	SECTION 1 IS NOT EMPTY	SETS	520
	IF(VSTARX.GT.VD(1, I5SEC).AND.VD(2, I5SEC).GE.VD(1, I5SEC))GO TO 310	SETS	530
	GO TO 320	SETS	540
C	SET SIGN 1	SETS	550
310	VSIGN(1, I5SEC)=VD(1, I5SEC)	SETS	560
	LSIGN(1)=ON	SETS	570
	NSON(1)= NSON(1)+1	SETS	580
	LMTAG(1) = VD(1, I5SEC)	SETS	590
	IF(CBUG1 .EQ. 1) WRITE(6, 902) VSIGN(1, I5SEC)	SETS	600
902	FORMAT(1X, 'SIGN 1 IS SET TO', F5.1)	SETS	610
C		SETS	620
C	LOOK FOR LOCAL MINIMUM	SETS	630
C	FIRST CHECKPOINT IS VD(NL+1)	SETS	640
C		SETS	650
320	LM=NL+1	SETS	660
	IF(CBUG1 .EQ. 1 .AND. LSIGN(1) .EQ. OFF) WRITE(6, 903)	SETS	670
903	FORMAT(1X, 'SIGN 1 WILL NOT BE SET')	SETS	680
C		SETS	690
C	DETERMINE LAST CHECKPOINT LMLAST	SETS	700
C		SETS	710
	LMLAST=NF-1	SETS	720
	IF(CARA.NE.1 .AND. NF.EQ.ITEN) LMLAST=NF	SETS	730
C		SETS	740

C	ARE THERE MORE DETECTORS TO BE CHECKED?	SETS 750
C		SETS 760
	IF(LMLAST-LM.LE.-1) GO TO 600	SETS 770
C		SETS 780
	IF(CBUG1 .EQ. 1) WRITE(6,904) LM	SETS 790
904	FORMAT(1X,'CHECK FOR LOCAL MINIMUM STARTING AT VD(0,I2,0)')	SETS 800
	IF(CBUG1 .EQ. 1) WRITE(6,905) LMLAST	SETS 810
905	FORMAT(1X,'LAST CHECKPOINT IS VD(0,I3,0) //')	SETS 820
C		SETS 830
C		SETS 840
C	CHECK FOR LOCAL MINIMA FROM STATIONS LM TO LMLAST	SETS 850
C		SETS 860
	DO 500 I=LM,LMLAST	SETS 870
	IF(I.EQ.LMLAST) GO TO 330	SETS 880
	IF(VD(I-1,I5SEC).GT.VD(I,I5SEC).AND.VD(I+1,I5SEC).GE.VD(I,I5SEC))	SETS 890
2	GO TO 340	SETS 900
	GO TO 500	SETS 910
C	CHECK FOR LOCAL MINIMUM FOR VD(LMLAST,I5SEC)	SETS 920
330	IF(VD(I,I5SEC).LT.VD(I-1,I5SEC)) GO TO 340	SETS 930
	GO TO 500	SETS 940
C		SETS 950
C	LOCAL MINIMUM FOUND -- SET SIGNS BACK TO SIGN I OR SIGN NL+1	SETS 960
C		SETS 970
340	NBEGIN=NL+1	SETS 980
	IF(LSIGN(1).EQ.ON) NBEGIN=1	SETS 990
C		SETS 1000
C		SETS 1010
	IF(CBUG1 .EQ. 1) WRITE(6,906) I,NBEGIN	SETS 1020
906	FORMAT(1X,'LOCAL MIN. FOUND AT VD(0,I3,0). SET SIGNS BACK TO SIGN	SETS 1030
	1',[4])	SETS 1040
	DO 400 J=NBEGIN,I	SETS 1050
	JJ=I+NBEGIN-J	SETS 1060
C		SETS 1070
C	CHECK LMTAG(JJ) --- WAS SIGN JJ PREVIOUSLY CONSIDERED	SETS 1080
C	DUE TO LOCAL MINIMUM OF SMALLER VALUE FURTHER UPSTREAM	SETS 1090
C		SETS 1100
	IF(LMTAG(JJ) .LT. VD(I,I5SEC)) GO TO 384	SETS 1110

C		SETS 1120
C		SETS 1130
C	SKIP SIGN SETTING AT LOCAL MINIMUM	SETS 1140
C		SETS 1150
	IF(J .EQ. NBEGIN) GO TO 385	SETS 1160
C		SETS 1170
C	LMTAG(JJ) NOT CHANGED FOR SIGN LOCATED AT LOCAL MINIMUM	SETS 1180
C		SETS 1190
	LMTAG(JJ) = VD(I, I5SEC)	SETS 1200
C		SETS 1210
C	CALCULATIONS FOR SETTING JJ-TH SIGN	SETS 1220
	IF(JJ.GT.1) VIM1=VD(JJ-1, I5SEC)	SETS 1230
	IF(JJ.EQ.1) VIM1=VSTARX	SETS 1240
C	COMPUTE INITIAL SPACING OVER THE JCARS VEHICLES	SETS 1250
	KK=I-JJ+1	SETS 1260
	S1=FLOAT(KK)*528.	SETS 1270
C	COMPUTE ESTIMATED FINAL SPACING	SETS 1280
	S2 = 0.	SETS 1290
	KJ = 0	SETS 1300
	DO 393 K=JJ, I	SETS 1310
	KJ=KJ+1	SETS 1320
	ND4=ND(K, I5SEC)	SETS 1330
	FND4=FLOAT(ND4)	SETS 1340
	VDK=VD(K, I5SEC)	SETS 1350
	IF(K .EQ. 1) VDM=VSTARX	SETS 1360
	IF(K .NE. 1) VDM=VD(K-1, I5SEC)	SETS 1370
	VKBAR=(VDK+VDM)*0.5	SETS 1380
	IF(ND4 .GT. 15 .OR. VDK .LT. 10.) GO TO 391	SETS 1390
	IF(ND4 .EQ. 0) GO TO 396	SETS 1400
	HDK=(528./FND4 - 20.)/VKBAR	SETS 1410
	IF(HDK.LT.HTEST)GO TO 341	SETS 1420
	HDK=HTEST	SETS 1430
	IF(CBUG1.EQ.1) WRITE(6,941) K	SETS 1440
941	FORMAT(' ', '*** HDK(' , I2, ') IS TOO LARGE... REPLACED BY HTEST')	SETS 1450
341	HDKK(KJ)=HDK	SETS 1460
	DK=(HDK*VD(I, I5SEC)+20.)*FND4	SETS 1470
	GO TO 392	SETS 1480

396	HDKK(KJ) = 0.	SETS 1490
	DK = 0.	SETS 1500
	GO TO 392	SETS 1510
391	DK=(HBAR*VD(I,15SEC)+20.)*FND4	SETS 1520
	HDKK(KJ)=HBAR	SETS 1530
392	S2=S2+DK	SETS 1540
393	CONTINUE	SETS 1550
C	COMPUTE OPTIMUM ACCELERATION ON SECTION JJ (NEGATIVE)	SETS 1560
	DV = VIM1-VD(I,15SEC)	SETS 1570
	IF(DV .LE. 0.) GO TO 374	SETS 1580
	DV2=DV*DV	SETS 1590
	DS = S2 - S1	SETS 1600
	IF(DS.GT.0.) GO TO 377	SETS 1610
	TSIGN = - DS * 2. / DV	SETS 1620
	AOPT =-DV/TSIGN	SETS 1630
C	COMPUTE SETTING FOR JJ-TH SIGN	SETS 1640
	SARG = VIM1*VIM1 + SGNC*AOPT	SETS 1650
	IF(SARG .GT. 0.) GO TO 375	SETS 1660
	VSJJ = 0.	SETS 1670
	GO TO 376	SETS 1680
374	IF(CBUG1 .EQ. 1) WRITE(6,930) JJ,I,DV	SETS 1690
930	FORMAT(9X,'SIGN',I4,' NOT SET SINCE DV=VIM1-VD(' ,I3,')=' ,F6.1,	SETS 1700
	2 ' IS NEGATIVE' //)	SETS 1710
	GO TO 400	SETS 1720
377	IF(CBUG1.EQ.1) WRITE(6,921) JJ,DS	SETS 1730
921	FORMAT(9X,'SIGN',I3,' NOT SET SINCE DS = S2 - S1 =',F5.1,	SETS 1740
	2 ' IS POSITIVE'//)	SETS 1750
	GO TO 400	SETS 1760
375	VSJJ = SQRT(SARG)	SETS 1770
376	CONTINUE	SETS 1780
	TMIN=528./VDM	SETS 1790
	IF(TSIGN.GT.TMIN) GO TO 397	SETS 1800
	WRITE(6,922) JJ,TNOW,TSIGN,TMIN,DV,DS,S1,AOPT,VSJJ	SETS 1810
922	FORMAT(/1X,125(' '),/1X,'SIGN',I3,3X,'TNOW=' ,F6.1,3X,'TSIGN=' ,	SETS 1820
	2 F5.1,' .LT. TMIN=' ,F5.1,3X,'DV=' ,F5.1,3X,'DS=' ,F5.0,	SETS 1830
	3 3X,'S1=' ,F6.0,3X,'AOPT=' ,F6.2,3X,'VSJJ(TENT.)=' ,F5.1,' FT/SEC')	SETS 1840
	VSJJ=VD(JJ,15SEC)	SETS 1850

	WRITE(6,923) JJ,VSJJ	SETS 1860
923	FORMAT(1X,'SET SIGN',I3,' TO',F5.1,' FT/SEC = VALUE FROM DETECTOR	SETS 1870
	2AT LOCATION OF SIGN'/1X,'125(' '*')/)	SETS 1880
397	IF(CBUG1 .EQ. 0) GO TO 2C2	SETS 1890
	JJM = JJ - 1	SETS 1900
	WRITE(6,915) JJ,JJM,VIM1,VD(1,15SEC),S1,S2,DS	SETS 1910
915	FORMAT(6X,'NOW SETTING SIGN',I3,' FOR IMAGINARY CAR LOCATED AT SIG	SETS 1920
	2N',I3,'/9X,'ASSUMED INITIAL VELOCITY =' ,F5.1,' AND ASSUMED FINAL V	SETS 1930
	3ELOCITY =' ,F5.1,'/11X,'INITIAL SPACING =' ,F6.0,' FEET AND'	SETS 1940
	4 ' FINAL SPACING =' ,F6.0,' FEET',5X,'S2 - S1 =' ,F6.0)	SETS 1950
	WRITE(6,916) (IA,HDKK(IA-JJ+1), IA=JJ,I)	SETS 1960
916	FORMAT((11X,7('H(' ,I2,')=' ,F4.2,3X)))	SETS 1970
	WRITE(6,917) DV,DV2,TSIGN,AOPT,SARG,VSJJ	SETS 1980
917	FORMAT(SETS 1990
	211X,'DV =' ,F6.2,' FT/SEC',5X,'DV*DV =' ,F7.2,' TSIGN=' ,F5.1,' SEC.'	SETS 2000
	3,5X,'AOPT =' ,F6.2,' FT/(SEC)**2',5X,'SARG =' ,F8.1/ 9X,'COMPUTED SI	SETS 2010
	4GN SETTING = VSJJ =' ,F6.1,' FT/SEC')	SETS 2020
202	CONTINUE	SETS 2030
	IF(VSJJ .LT. VIM1) GO TO 380	SETS 2040
	IF(CBUG1 .EQ. 1) WRITE(6,911)	SETS 2050
911	FORMAT(6X,'SIGN NOT SET BECAUSE COMPUTED SETTING IS GREATER THAN S	SETS 2060
	2PEED OF CAR 528 FEET UPSTREAM' //)	SETS 2070
	GO TO 400	SETS 2080
380	IF(VSJJ .LT. VSIGN(JJ,15SEC)) GO TO 381	SETS 2090
	IF(CBUG1 .EQ. 1) WRITE(6,912)	SETS 2100
912	FORMAT(6X,'SIGN NOT SET BECAUSE COMPUTED SETTING IS GREATER THAN P	SETS 2110
	2REVIOUS SETTING' //)	SETS 2120
	GO TO 400	SETS 2130
381	IF(ND(JJ,15SEC) .NE. 0) GO TO 382	SETS 2140
	IF(CBUG1 .EQ. 1) WRITE(6,913) JJ	SETS 2150
913	FORMAT(6X,'SIGN NOT SET BECAUSE THERE ARE NO CARS ON SECTION',I3 /	SETS 2160
	2/)	SETS 2170
	GO TO 400	SETS 2180
385	IF(CBUG1 .EQ. 1) WRITE(6,920)	SETS 2190
920	FORMAT(1X,'SKIP SETTING OF SIGN LOCATED AT LOCAL MINIMUM'//)	SETS 2200
	GO TO 400	SETS 2210
384	IF(CBUG1 .EQ. 1) WRITE(6,918) JJ,LMTAG(JJ)	SETS 2220

918	FORMAT(6X,'SIGN',I3,' HAS BEEN PREVIOUSLY CONSIDERED DUE TO LOCAL	SETS 2230
	2MINIMUM FURTHER UPSTREAM OF SMALLER VALUE, I.E.,I4,' FT/SEC' //	SETS 2240
3	1X,'**STOP SCANNING SIGNS AND LOCK FOR OTHER LOC. MINIMA FURTHER	SETS 2250
	4 DOWNSTREAM' //)	SETS 2260
	GO TO 500	SETS 2270
C	SET JJ-TH SIGN	SETS 2280
382	VSIGN(JJ,I5SEC)=VSJJ	SETS 2290
	IF(CBUG1 .EQ. 0) GO TO 383	SETS 2300
	IF(LSIGN(JJ) .EQ. OFF) WRITE(6,908) JJ,VSJJ	SETS 2310
	IF(LSIGN(JJ) .EQ. ON) WRITE(6,909)JJ,VSJJ	SETS 2320
908	FORMAT(11X,'SET SIGN',I3,' TO',F6.1//)	SETS 2330
909	FORMAT(11X,'RE-SET SIGN',I3,' TO',F6.1//)	SETS 2340
383	IF(I.NE. JJ .OR. VSJJ.GT.VD(JJ,I5SEC)-2.) GO TO 203	SETS 2350
	VSIGN(JJ,I5SEC)=VD(JJ,I5SEC)-2.	SETS 2360
	IF(CBUG1.EQ.1) WRITE(6,931) JJ,VSIGN(JJ,I5SEC)	SETS 2370
931	FORMAT(11X,'SIGN',I4,' IS AT LOCAL MINIMUM. RE-SET SIGN TO',	SETS 2380
	2 F6.1//)	SETS 2390
203	CONTINUE	SETS 2400
	IF(LSIGN(JJ) .EQ. ON) GO TO 400	SETS 2410
	LSIGN(JJ)=ON	SETS 2420
	NSON(JJ) = NSON(JJ)+1	SETS 2430
400	CONTINUE	SETS 2440
500	CONTINUE	SETS 2450
600	DO 700 I=1,ITEN	SETS 2460
	IF(ND(I,I5SEC) .EQ. 0) GO TO 700	SETS 2470
	NSFULL(I)= NSFULL(I)+1	SETS 2480
700	CONTINUE	SETS 2490
	IF(CRCUND .EQ. 0) GO TO 950	SETS 2500
C		SETS 2510
C	ROUND SIGN SETTING -- POSSIBLY SET SOME OFF	SETS 2520
C		SETS 2530
	IF(CBUG1 .EQ. 1) WRITE(6,801)	SETS 2540
801	FORMAT(1X,'RESULTS OF ROUNDING'//)	SETS 2550
	DO 720 I=1,ITEN	SETS 2560
	VSIG=VSIGN(I,I5SEC)	SETS 2570
	IF(LSIGN(I) .EQ. OFF) GO TO 720	SETS 2580
	IF(I .EQ. 1) VIM=VSTARX	SETS 2590

```

IF( I .NE. 1 ) VIM=VD(I-1,I5SEC)
IF( VIM - VSIG .GT. 3.666667 ) GO TO 710
VD-VSIGN IS SMALL ---- SET SIGN I OFF
C
705 LSIGN(I)=OFF
VSN(I,I5SEC)=9.E10
NSON(I)=NSON(I)-1
IF( CBUG1.EQ.1 ) WRITE(6,702) I
702 FORMAT(IX,'VIM - VSIGN IS LESS THAN 3.666667 ---- SET SIGN',I4,
2 ' OFF')
GO TO 720
C
710 ROUND SIGN I
VSN(I,I5SEC) = FLOAT( IFIX(VSIG/Y*.5) ) * Y
IF( CBUG1.EQ.1 ) WRITE(6,703) I, VSN(I,I5SEC)
703 FORMAT(IX,'SIGN',I4,' ROUNDED TO',F6.1)
IF( VIM-VSIGN(I,I5SEC) .LT. 3.666667 ) GO TO 705
720 CONTINUE
C
C
950 CONTINUE
CHECK WHETHER SIGN SEEN BY CARZ SHOULD BE SET
IF( LSIGN(NL).EQ. ON ) GO TO 1000
IF( NL.EQ. ITEN ) GO TO 1000
IF( LSIGN(LM).EQ. OFF ) GO TO 1000
VSN(NL,I5SEC) = VSN(LM,I5SEC)
LSIGN(NL) = ON
NSON(NL) = NSCN(NL) + 1
IF( CBUG1.EQ. 1 ) WRITE(6,831) VSN(NL,I5SEC),LM
831 FORMAT( /IX,'SET SIGN SEEN BY LAST CAR ON ROAD TO',F6.1,'. SAME A
15 SIGN',I4//)
1000 CONTINUE
RETURN
END
SETS 2600
SETS 2610
SETS 2620
SETS 2630
SETS 2640
SETS 2650
SETS 2660
SETS 2670
SETS 2680
SETS 2690
SETS 2700
SETS 2710
SETS 2720
SETS 2730
SETS 2740
SETS 2750
SETS 2760
SETS 2770
SETS 2780
SETS 2790
SETS 2800
SETS 2810
SETS 2820
SETS 2830
SETS 2840
SETS 2850
SETS 2860
SETS 2870
SETS 2880
SETS 2890
SETS 2900
SETS 2910

```

```

SUBROUTINE STEPH(XIN,I1,I2,FT1)
C      XIN = INPUT IN SECONDS
C      I1 = # OF MINUTES
C      I2 = # OF 10 SECONDS
C      FT1 = SECONDS & FRACTION OF SECOND

I1= XIN / 60. +.0001
F = XIN - 60.*FLOAT(I1)
I2 = F / 10.
FT1= F - 10.*FLOAT(I2)
IF( FT1 .LE. 0.0 ) FT1 = +0.0
IF(FT1.LT. 9.95) GO TO 111
FT1 = 0.0
I2 = I2 + 1
IF(I2 .LE. 5) GO TO 111
I2 = 0
I1 = I1 + 1
111 RETURN
END

```

```

STEP 10
STEP 20
STEP 30
STEP 40
STEP 50
STEP 60
STEP 70
STEP 80
STEP 90
STEP 100
STEP 110
STEP 120
STEP 130
STEP 140
STEP 150
STEP 160
STEP 170
STEP 180
STEP 190

```

	SUBROUTINE F3	F3	10
	IMPLICIT INTEGER (C,I-N)	F3	20
	INTEGER*2 JSEC, JVSET, IVSEC(100,5), CECT, IBRNCH, COMPLY	F3	30
	INTEGER ON, OFF	F3	40
	COMMON XCAR(3861), VCAR(3861), ANOW(350), CYCLE1, CARONE, TNOW, CARA,	F3	50
	1 CARZ, CYCLEZ, CYCLEA	F3	60
	COMMON /AAAAAA/ NCARS, NMILE, VSTART, TXPRNT, IBRNCH(350),	F3	70
	1 CARNOW, NOPRNT, CONTRL, ON, OFF, BETA, TAUS, KSIGN, LSIGN(100),	F3	80
	2 HBAR, HDRV(350), COMPLY(350), VSIGN(100,7)	F3	90
	COMMON /BBBBBB/ TF3(5), AF3(4), BASE, NREP, NREPI, VI3	F3	100
	COMMON /GGGGGG/ ISIGN, JSEC, JVSET, IVSEC, IVP, ITENP, CECT(350)	F3	110
C		F3	120
C	THIS SUBROUTINE UPDATES THE CARONE-TH CAR	F3	130
C	I.E. THE FURTHEST CAR ON HIGHWAY OR EXTENSION	F3	140
C		F3	150
C	LEAD CAR GOES ACCORDING TO CONSTANTS TF3(I) AND AF3(I)	F3	160
C	TNOW REPRESENTS OLD TIME	F3	170
C	COMPUTED VALUE OF 'ACC' REPRESENTS ACCEL. AT TNOW	F3	180
C	COMPUTED VALUES OF 'XCAR' & 'VCAR' ARE FOR T=TNOW+.2	F3	190
C		F3	200
	IF(CARONE .NE. 1) GO TO 60	F3	210
50	IF(TNOW.GT.TF3(1) .AND. NREPI.NE.NREP) GO TO 100	F3	220
60	ANOW(CARONE)=0.	F3	230
	GO TO 600	F3	240
100	IF(TNOW .GT. TF3(2)+BASE) GO TO 200	F3	250
	ANOW(CARONE)=AF3(1)	F3	260
	GO TO 600	F3	270
200	IF(TNOW .GT. TF3(3)+BASE) GO TO 300	F3	280
	ANOW(CARONE)=AF3(2)	F3	290
	GO TO 600	F3	300
300	IF(TNOW .GT. TF3(4)+BASE) GO TO 400	F3	310
	ANOW(CARONE)=AF3(3)	F3	320
	GO TO 600	F3	330
400	IF(TNOW .GT. TF3(5)+BASE) GO TO 500	F3	340
	ANOW(CARONE)=AF3(4)	F3	350
	GO TO 600	F3	360
500	NREPI = NREPI + 1	F3	370

	BASE = BASE + TF3(5) - TF3(1)	F3	380
	GO TO 50	F3	390
C	UPDATE XCAR AND VCAR	F3	400
600	VCAR(CYCLE1) = VCAR(CYCLE1+1) + ANOW(CARONE)*0.2	F3	410
	XCAR(CYCLE1) = XCAR(CYCLE1+1) + VCAR(CYCLE1+1)*0.2 + ANOW(CARONE)*	F3	420
2	0.02	F3	430
C		F3	440
C	UPDATE DETECTORS	F3	450
C		F3	460
	CALL NDVD	F3	470
C		F3	480
	IF(JVSET .EQ. 0.OR. CARONE .GT. 100) RETURN	F3	490
	IVSEC(CARONE,JSEC) = MFIX(VCAR(CYCLE1)+0.5)	F3	500
	RETURN	F3	510
	END	F3	520

C C C C	SUBROUTINE NDVD UPDATE ND AND VD -- DETECTOR INFORMATION THIS SUBROUTINE IS CALLED BY F3 IMPLICIT INTEGER (C,I-N) INTEGER*2 ND, CECT,JSEC,JVSET,IVSEC(100,5) COMMON XCAR(3861),VCAR(3861),ANOM(350),CYCLE1,CARONE,TNOW,CARA, 2 CARZ,CYCLEZ,CYCLEA COMMON /GGGGGG/ ISIGN,JSEC,JVSET,IVSEC,IVP,ITENP,CECT(350) COMMON/NNNNNN/ITEN,ITENM,ITEND2,ND(100,7),I5SEC,VD(100,7) C1 = CECT(CARONE) C2=XCAR(CYCLE1)/528. + 1. IF(C2 .EQ. C1) GO TO 601 CECT(CARONE) = C2 IF(C2 .GT. ITENP) GO TO 601 IF(C2 .EQ. 1) GO TO 600 ND(C1,I5SEC) = ND(C1,I5SEC) - 1 VD(C1,I5SEC) = VCAR(CYCLE1) IF(C2 .EQ. ITENP) GO TO 601 ND(C2,I5SEC) = ND(C2,I5SEC) + 1 600 CONTINUE 601 RETURN END	NDVD 10 NDVD 20 NDVD 30 NDVD 40 NDVD 50 NDVD 60 NDVD 70 NDVD 80 NDVD 90 NDVD 100 NDVD 110 NDVD 120 NDVD 130 NDVD 140 NDVD 150 NDVD 160 NDVD 170 NDVD 180 NDVD 190 NDVD 200 NDVD 210 NDVD 220 NDVD 230 NDVD 240
------------------	--	---

	SUBROUTINE FINISH	FINI	10
	IMPLICIT INTEGER (C,I-N)	FINI	20
	INTEGER*2 JSTOP, JEXIT, ND, COMPLY	FINI	30
	INTEGER*2 IDXOUT(350), IVOUT(350), IBRNCH, NSCN, NSFULL	FINI	40
	INTEGER ON, OFF, ENTER	FINI	50
	COMMON /AAAAAA/ NCARS, NMILE, VSTART, TXPRNT, IBRNCH(350),	FINI	60
	1 CARNOW, NOPRNT, CONTAL, ON, OFF, BETA, TAUS, KSIGN, LSIGN(100),	FINI	70
	2 HBAR, HDRV(350), COMPLY(350), VSIGN(100, 7)	FINI	80
	COMMON /BBBBBB/TF3(5), AF3(4), BASE, NREP, NREPI, VI3	FINI	90
	COMMON /CCCCC/CBUG1, CROUND, SGNC	FINI	100
	COMMON /FFFFFF/TOTAL, ATOTAL(350), TRAVEL(350), JEXIT(350),	FINI	110
	1 JSTOP(350), IDXOUT, IVOUT	FINI	120
	COMMON /KKKKK/ESIG2, ETSTGP, ETOTLT, EPER, EPCMP, IEVAV1, IEOX, NENS,	FINI	130
	2 JENS, EQ, ESPACE(10), RRI	FINI	140
	COMMON /MMMMM/IVPRNT, TV1, TV2, IFIFTH, IFOL, SPACE(10), TSPACE(10)	FINI	150
	COMMON /NNNNN/ITEN, ITEMN, ITEND2, ND(100, 7), I5SEC, VD(100, 7)	FINI	160
	COMMON /UUUUU/ NSON(100), NSFULL(100), NFT(100), HTEST, VSTARX	FINI	170
	COMMON /ZZZZZ/ ENTER, QHOUR, HMIN, HMODE, IR, HBAR2, PCMPY, PCMP	FINI	180
		FINI	190
	IF(NENS.GT.1)WRITE(6,498) JENS, NENS	FINI	200
498	FORMAT(1H1, 'THIS RUN IS NUMBER', I2, ' OF AN ENSEMBLE OF', I2, ' RUNS'	FINI	210
	2)	FINI	220
	IF(NENS.EQ.1) WRITE(6,499)	FINI	230
499	FORMAT(1H1)	FINI	240
	WRITE(6,500) NCARS, NMILE	FINI	250
500	FORMAT(1X, 'THIS RUN IS MADE WITH', I4, ' CARS FOR', I3, ' MILES')	FINI	260
	WRITE(6,503)	FINI	270
503	FORMAT(/1X, 'THE LEAD CAR UNDERGOES REPETITIVE MANEUVERS USING SUBR	FINI	280
	ROUTINE F3')	FINI	290
	IF(ENTER .EQ. 1) GO TO 505	FINI	300
	PARMX=HBAR *VSTART+20.	FINI	310
	PARMT=HBAR+20./VSTART	FINI	320
	PARMQ=VSTART/PARMX*3600.	FINI	330
	QQ=PARMQ	FINI	340
	WRITE(6,504) ENTER	FINI	350
504	FORMAT(/1X, 'ENTER=', I2)	FINI	360
	WRITE(6,504) PARMX, PARMT, PARMQ	FINI	370

```

504 FORMAT( 1X, 'CARS ENTER THE HIGHWAY AT EQUAL SPACING OF', F6.1, FINI 380
2 ' FEET (TIME HEADWAY =', F6.2, ' SEC), CORRESPONDING TO A FLOW OF' FINI 390
3 , F6.0, ' VEH/HR' ) FINI 400
GO TO 507 FINI 410
505 TBAR2=HBAR2+20./VSTART FINI 420
QBAR2=3600./TBAR2 FINI 430
QQ=QBAR2 FINI 440
PARMX2=HBAR2*VSTART+20. FINI 450
WRITE(6,506) HBAR2,TBAR2,PARMX2,QBAR2 FINI 460
506 FORMAT(//1X, 'CARS ENTER THE HIGHWAY WITH RANDOM SPACINGS ( HBAR2 = FINI 470
2', F5.2, ' ) WITH AVERAGE TIME HEADWAY TBAR2 =', F5.2, ' , AVERAG FINI 480
3E SPACING OF', F6.1, ' FEET' / 1X, 'CORRESPONDING TO A FLOW OF QB FINI 490
4AR2 =', F6.0, ' VEH/HR' ) FINI 500
507 IF( CONTRL .EQ. 1 ) GO TO 509 FINI 510
WRITE(6,508) CONTRL FINI 520
508 FORMAT(//1X, 'CONTRL =', I2, ' THIS RUN IS WITHOUT SIGN CONTROL' ) FINI 530
GO TO 514 FINI 540
509 WRITE(6,5091) CONTRL FINI 550
5091 FORMAT(//1X, 'CONTRL =', I2, ' THIS RUN IS WITH SIGN CONTROL' ) FINI 560
IF( CROUND .EQ. 0 ) WRITE(6,510) CROUND FINI 570
510 FORMAT(//1X, 'CROUND =', I2, 3X, 'EXACT SIGN SETTINGS USED --- NO R FINI 580
2OUNDING' ) FINI 590
IF( CROUND .NE. 0 ) WRITE(6,511) CROUND,CROUND FINI 600
511 FORMAT(//1X, 'CROUND =', I2, 3X, 'SIGNS ROUNDED USING ALGORITHM', I2) FINI 610
WRITE(6,517) SGNC FINI 620
517 FORMAT(//1X, 'SGNC =', F7.1, 3X, 'VALUE OF CONSTANT USED TO COMPUTE FINI 630
2SIGN SETTINGS' ) FINI 640
514 CONTINUE FINI 650
C FINI 660
TAVG = TOTALT / NCARS FINI 670
IDXOUT(1) = 0 FINI 680
ISUMV = IVOUT(1) FINI 690
ISUMDX = IDXOUT(1) FINI 700
DC 803 I = 2, NCARS FINI 710
ISUMV = ISUMV + IVOUT(I) FINI 720
803 ISUMDX = ISUMDX + IDXOUT(I) FINI 730
IVAVG = ISUMV/NCARS FINI 740

```

```

ZIG=IVAVG
ZIG=ZIG*3600./5280.
IVAVG1=ZIG
IDXAVG = ISUMDX/(NCARS-1)
WRITE(6,611)
611 FORMAT(1H1,65X,'STOPPED',10X,'SPEED AT EXIT',8X,'SPACING'
1 ,7X,'HDRV'
2/67X,'TIME',12X,'FT/SEC MI/HR',7X,'EXIT',3X,'ENTER'/)
TSTOPT=0.
DO 804 I = 1,NCARS
CALL STEPH(TRAVEL(I),IT1,IT11,FT1)
JEXI = JEXIT(I)
TEXIT = FLOAT(JEXI )/5.
CALL STEPH(TEXIT ,IT2,IT22,FT2)
JSTO = JSTOP(I)
TSTOP = FLOAT(JSTO )/5.
TSTOPT=TSTOPT+TSTOP
CALL STEPH(TSTOP ,IT3,IT33,FT3)
ZT4 = IVOUT(I)
IT4 = (FIX( ZT4*3600./5280.*.5 )
HEADIN = HDRV(I)*VSTART + 20.
804 WRITE(6,607) I, IT1, IT11, FT1 ,IT2, IT22, FT2, IT3, IT33, FT3,
1 IVOUT(I),IT4,IDXOUT(I),HEADIN,HDRV(I)
607 FORMAT(' CAR(',I2,') TCOK',I3,1H-,I1,F3.1,8X,'TIME OF EXIT =',I3,
11H-,I1,F3.1,13X,I3,1H-,I1,F3.1,9X,I6 ,3X,I5 ,5X,I6 ,F7.0,F8.2)
CALL STEPH(TOTALT,IT1,IT11,FT1)
CALL STEPH(TAVG,IT2,IT22,FT2)
IT11 = 10*IT11 + FT1
FT2 = 10*IT22 + FT2
WRITE(6,602) IT1, IT11, IT2, FT2, IVAVG, IVAVG1, IDXAVG, TSTOPT
602 FORMAT(//' TOTAL TRAVEL TIME =',I5,' MIN.',I3,' SEC.'//' AVERAGE T FINI 1050
1R AVEL TIME =',I4,' MIN.',F5.1,' SEC.'//' AVERAGE SPEED AT EXIT =', FINI 1060
2I6 ,' FT/SEC =',I6 ,' MI/HR'//' AVERAGE SPACING AT EXIT =',I6 FINI 1070
3 ,' FT.'//' TOTAL STOPPED TIME =',F6.1,' SECONDS') FINI 1080
WRITE(6,605) FINI 1090
605 FORMAT(1H1,10X,'ACCELERATION',20X,'STANDARD DEVIATION'/14X,'NOISE' FINI 1100
1//12X,'SIGMA**2',28X,'SIGMA'/9X,'(FT/SEC-SEC)**2',14X,'(FT/SEC-SEC FINI 1110

```

	2) ,3X, (UNITS OF G) /)	FINI 1120
	TSIG2 = 0.	FINI 1130
	DO 605 I = 2, NCARS	FINI 1140
	JSTO = JSTOP(I)	FINI 1150
	TSTOP = FLOAT(JSTO) / 5.	FINI 1160
	JEXI = JEXIT(I)	FINI 1170
	TEXIT = FLOAT(JEXI) / 5.	FINI 1180
	SIGMA2 = .2 / (TEXIT - TF3(1) - TSTOP) * TOTAL(I)	FINI 1190
	SIGMA = SQRT(SIGMA2)	FINI 1200
	GS = SIGMA / 32.174	FINI 1210
	TSIG2 = TSIG2 + SIGMA2	FINI 1220
	WRITE(6,606) I, SIGMA2, SIGMA, GS	FINI 1230
606	FORMAT(' CAR', I3, F12.3, F29.4, F14.3)	FINI 1240
805	CONTINUE	FINI 1250
	AVSIG2 = TSIG2 / FLOAT(NCARS - 1)	FINI 1260
	AVSIG = SQRT(AVSIG2)	FINI 1270
	AVSIG1 = AVSIG / 32.174	FINI 1280
	WRITE(6,608) TSIG2, AVSIG2, AVSIG, AVSIG1	FINI 1290
608	FORMAT('// TSIG2 = TOTAL ACC. NOISE (EXCLUDING CAR 1) =', F8.3, ' (F	FINI 1300
	IT/SEC-SEC)**2' // AVSIG2 = TSIG2 / (NCARS - 1) = AVER. ACC. NOISE =',	FINI 1310
	2F7.3, ' (FT/SEC-SEC)**2' // AVSIG = SQRT(AVSIG2) = STANDARD DEVIATI	FINI 1320
	ON OF ACC. NOISE =', F6.3, ' FT/SEC-SEC =', F5.3, ' G')	FINI 1330
	WRITE(6,906)	FINI 1340
906	FORMAT(1H1, 'AVERAGE SPACING FOR EVERY TENTH VEHICLE' //)	FINI 1350
	IND = NCARS / 10	FINI 1360
	DO 610 JSP = 1, IND	FINI 1370
	SPACE(JSP) = SPACE(JSP) / TSPACE(JSP)	FINI 1380
	JCAR = JSP * 10	FINI 1390
	WRITE(6,907) JCAR, SPACE(JSP)	FINI 1400
907	FORMAT(3X, 'CAR', I4, 3X, 'AV. SPACING =', F7.2, ' FT.')	FINI 1410
610	ESPACE(JSP) = ESPACE(JSP) + SPACE(JSP)	FINI 1420
	IF(CONTRL .EQ. 0) GO TO 704	FINI 1430
	WRITE(6,900)	FINI 1440
900	FORMAT(1H1, 23X, 'DUTY CYCLE FOR ADVISORY SIGNS' // 2X, 'DUTY CYCLE	FINI 1450
	2= 100 * (TIME SIGN K IS ON) / (TIME SECTION K IS NON-EMPTY)' ///	FINI 1460
	3 17X, 'NUMBER OF 5 SEC. INTERVALS' /	FINI 1470
	4 10X, 'SIGN', 6X, 'SIGN', 8X, 'SECTION IS', 5X, 'DUTY CYCLE SIGN' /	FINI 1480

5	9X, 'NUMBER	IS ON	NON-EMPTY	(PERCENT)	NUMBER'//)	FINI	1490
	INN1=0					FINI	1500
	INN2=0					FINI	1510
	DO 700 I=1,ITEN					FINI	1520
	IN1= NSON(I)					FINI	1530
	IN2= NSFULL(I)					FINI	1540
	PER= FLOAT(IN1)/FLOAT(IN2) * 100.					FINI	1550
	INN1=INN1+IN1					FINI	1560
	INN2=INN2+IN2					FINI	1570
700	WRITE(6,901) I, IN1, IN2, PER, I					FINI	1580
901	FORMAT(11X, I3, 6X, I3, 11X, I3, 10X, F6.1, 7X, I3)					FINI	1590
	PER=FLOAT(INN1)/FLOAT(INN2)*100.					FINI	1600
	WRITE(6,902) PER					FINI	1610
902	FORMAT(///1X, '(SIGN-ON TIME) / (SECTION NON-EMPTY TIME) * 100 =',					FINI	1620
	2 F6.1, ' (PERCENT)')					FINI	1630
	WRITE(6,903)					FINI	1640
903	FORMAT(1H1,40X, 'COMPLIANCE FACTORS'/6CX, 'FACTOR=1 CAR FOLLOWS SI					FINI	1650
	2GN'/60X, 'FACTOR=0 CAR IGNORES SIGN'//1X,5E'CAR NO. FACTOR',7X)					FINI	1660
	3 //)					FINI	1670
	WRITE(6,904) (I,COMPLY(I),I=2,NCARS)					FINI	1680
904	FORMAT((3X,5(I3,7X,I1,12X)))					FINI	1690
	WRITE(6,905) PCMPY,PCMP					FINI	1700
905	FORMAT(///1X, 'PCMPY =',F5.3, ' DESIRED FRACTION OF COMPLYING CAR					FINI	1710
	2S' / 1X, 'PCMP =',F5.3, ' ACTUAL FRACTION OF COMPLYING CARS')					FINI	1720
704	IF(NENS .EQ. 1) RETURN					FINI	1730
	ESIG2 = ESIG2 + AVSIG2					FINI	1740
	ETSTCP = ETSTCP + TSTGPT					FINI	1750
	ETOTLT = ETOTLT + TCTALT					FINI	1760
	EQ = EQ + QQ					FINI	1770
	IEVAV1 = IEVAV1 + IVAVG1					FINI	1780
	EPER = EPER + PER					FINI	1790
	EPCMP = EPCMP + PCMP					FINI	1800
	IEDX = IEDX + IDXAVG					FINI	1810
	IF(JENS .LT. NENS) RETURN					FINI	1820
C	COMPUTE ENSEMBLE AVERAGES					FINI	1830
	FNENS = FLOAT(NENS)					FINI	1840
	ESIG2 = ESIG2/FNENS					FINI	1850

ETSTOP = ETSTOP/FNENS	FINI 1860
ETOTLT = ETOTLT/FNENS	FINI 1870
EQ = EQ/FNENS	FINI 1880
IEVAV1 = FLOAT(IEVAV1)/FNENS + 0.5	FINI 1890
EPER = EPER/FNENS	FINI 1900
EPCMP = EPCMP/FNENS	FINI 1910
IEDX = FLOAT(IEDX)/FNENS + 0.5	FINI 1920
C WRITE HEADING DESCRIBING ENSEMBLE OF RUNS	FINI 1930
WRITE(6,699) NENS,NCARS,NMILE, ENTER,EQ,CONTRL	FINI 1940
699 FORMAT(1H1,20X,'ENSEMBLE STATISTICS FOR',I2,' RUNS'//	FINI 1950
2 1X,'NCARS=',I4 / ' NMILE=',I3 / ' SUBROUTINE F3',	FINI 1960
3 ' USED FOR LEAD CAR' / ' ENTER=',I2 /	FINI 1970
4 ' Q=',F6.0 / ' CONTRL=',I2)	FINI 1980
WRITE(6,707)	FINI 1990
707 FORMAT(//' AV. SPACING FOR EVERY TENTH CAR')	FINI 2000
DO 705 I=1,IND	FINI 2010
JCAR = I*10	FINI 2020
ESPACE(I) = ESPACE(J)/FNENS	FINI 2030
WRITE(6,706) JCAR,ESPACE(I)	FINI 2040
706 FORMAT(5X,'CAR',I4,' AV. SPACING =',F7.2)	FINI 2050
705 CONTINUE	FINI 2060
IF(CONTRL.EQ.1) WRITE(6,701) SGNC	FINI 2070
701 FORMAT(/1X,'SGNC=',F6.0)	FINI 2080
C WRITE ENSEMBLE AVERAGES	FINI 2090
WRITE(6,702) ESIG2,ETSTOP,ETOTLT,IEVAV1,IEDX	FINI 2100
702 FORMAT(///10X,'ENSEMBLE AVERAGES' //1X,'SIGMA SQUARED=',F6.3//	FINI 2110
2 1X,'TOTAL STOPPED TIME =',F6.1,' SEC.'//	FINI 2120
3 1X,'TOTAL TRAVEL TIME =',F7.1,' SEC.'//	FINI 2130
4 1X,'AV. SPEED AT EXIT =',I4,' MI/HR' //	FINI 2140
5 1X,'AV. SPACING AT EXIT =',I4,' FT.' //)	FINI 2150
IF(CONTRL .EQ. 0) RETURN	FINI 2160
WRITE(6,703) EPER, EPCMP	FINI 2170
703 FORMAT(1X,'SIGN DUTY CYCLE=',F5.1,' PER-CENT' //	FINI 2180
2 1X,'COMPLIANCE FACTOR=',F6.3)	FINI 2190
RETURN	FINI 2200
END	FINI 2210

REFERENCES

- 1 "An Inventory of Freeway Surveillance and Operational Control Activities," HRB, Highway Research Circular, No. 108, June 1970.
- 2 Edie, L.C. and R.S. Foote, "Traffic Flow in Tunnels," Proc. Highway Research Board, Vol. 37, pp.334-344, 1958.
- 3 Edie, L.C. and R.S. Foote, "Experiments in Single Lane Flow in Tunnels," The Theory of Traffic Flow, R. Herman, Ed., Elsevier, 1961.
- 4 Edie, L.C. and R.S. Foote, "Effect of Shock Waves on Tunnel Traffic Flow," HRB Proceedings, Vol.39, pp.492-505, 1960.
- 5 Foote, R.S. and D.C. Gazis, "Surveillance and Control of Tunnel Traffic by an On-Line Digital Computer," presented at the 32nd National Meeting, Operations Research Society of America, November 1967.
- 6 Pinnel, C., D.R. Drew, W.R. McCasland, and J. Wattleworth, "Evaluation of Entrance Ramp Control on a Six-Mile Freeway Section," HRR 157, pp.22-76, 1967.
- 7 Buhr, J.H., W.R. McCasland, J.D. Carvell, and D.R. Drew, "Design of Freeway Entrance Ramp Merging Control Systems," Research Report 504-3, Texas Transportation Institute, 1968. Also in Highway Research Record, No.279, pp.137-149, 1969.
- 8 Wiener, R.S., "Some Theoretical Considerations in the Design and Operation of Gap Acceptance Ramp Control Systems for Urban Freeways," Doctoral Dissertation, Polytechnic Institute of Brooklyn, June 1968.
- 9 Yagoda, H.N. and L. Pignataro, "The Analysis and Design of Freeway Entrance Ramp Control Systems," HRR 303, pp.56-73, 1970.
- 10 May, A.D., P. Athol, W. Parker, and J.B. Rudden, "Development and Evaluation of Congress Street Expressway Pilot Detection System," HRR 21, 1963.
- 11 May, Jr., Adolf D., "Experimentation with Manual and Automatic Ramp Control," Highway Research Record, No.59, pp. 9-38, 1964.

- 12 McDermott, J. M. , "Operational Effects of Automatic Ramp Control on Network Traffic," Highway Research Record, No.202, pp.1-25, 1967.
- 13 Pinnel, C. , D.R. Drew, W.R. McCasland, and J.A. Wattleworth, "Inbound Gulf Freeway Ramp Control Study I," Texas Transportation Inst. Res. Report 24-10, 1964.
- 14 Wattleworth, J.A. and C.E. Wallace, "Evaluation of the Operational Effects of an 'On-Freeway' Control System," Research Report 488-2, Texas Transportation Institute, November 1967.
- 15 Wattleworth, J.A. and J.D. Carvell, Jr. , "An Evaluation of Two Types of Freeway Control Systems," Research Report 488-6, Texas Transportation Institute, April 1968.
- 16 Michaels, R.M. , "Tension Responses of Drivers Generated on Urban Streets," HRB Bull.271, pp.29-44, 1960.
- 17 Michaels, R.M. and D. Solomon, "Effect of Speed Change Information on Spacing Between Vehicles," HRB Bull.330, pp.26-39, 1962.
- 18 Jones, T.R. and R.B. Potts, "The Measurement of Acceleration Noise -- A Traffic Parameter," Op.Res. , Vol.10, No.6, pp.745-763, 1962.
- 19 Von Stein, W. , "Traffic Flow with Pre-Signals and the Signal Funnel," The Theory of Traffic Flow, R. Herman, Ed. , Elsevier, 1961.
- 20 Drew, D.R. , K.A. Brewer, J.H. Buhr, and R.H. Whitson, "Multi-level Approach to the Design of a Freeway Control System," HRR 279, pp.40-55, 1969.
- 21 Whitson, R.H. , J.H. Buhr, D.R. Drew, and W.R. McCasland, "Real-time Evaluation of Freeway Quality of Traffic Service," HRR 289, pp.38-50, 1969.
- 22 Montroll, E.W. , "Acceleration Noise and Clustering Tendency of Vehicular Traffic," Theory of Traffic Flow, R. Herman, Ed. , Elsevier, 1961.
- 23 Helly, W. , "Simulation of Bottlenecks in Single-Lane Traffic Flow," Theory of Traffic Flow, R. Herman, Ed. , Elsevier, 1961.
- 24 Dare, C.E. , "Development of an Advisory Speed Signal System for High-Speed Intersections under Traffic-Actuated Control," HRR 286, pp.1-17, 1969.

- 25 Bierley, R.L. and J. Parkinson, "The Traffic Pacer System," HRR 49, pp.107-126, 1964.
- 26 Morrison, H.M., A.F. Underwood, and R.L. Bierley, "Traffic Pacer," HRB 338, pp.40-68, 1962.
- 27 Schmarsel, P., "Techniques for Improving Traffic Control," Traffic Engineering and Control, Vol.6, pp.361-364, 368, October 1964.
- 28 Reuschel, A., "Fahrzeugbewegungen in der Kolonne bei gleichförmig beschleunigtem oder verzögertem Leitfahrzeug," Zeits. d. Oesterreich. Ing. u. Arch. Vereines, Vol.95, 1950, pp.59-62, pp.73-77, Oesterreich. Ing. Archiv. 4, pp.193-215, 1950.
- 29 Pipes, L.A., "A Proposed Dynamic Analogy of Traffic," J.Ap.Phys. 24, pp.274-281, 1953.
- 30 Chandler, R.E., R. Herman, and E.W. Montroll, "Traffic Dynamics: Studies in Car Following," Operations Research, Vol.6, pp.165-184, 1958.
- 31 Kometani, E. and T. Sasaki, "On the Stability of Traffic Flow," J. Operations Research of Japan, 2, pp.11-26, 1958.
- 32 Kometani, E. and T. Sasaki, "A Safety Index for Traffic with Linear Spacing," Operations Research, Vol.7, pp.704-720, 1959.
- 33 Herman, R., E.W. Montroll, R.B. Potts, and R.W. Rothery, "Traffic Dynamics: Analysis of Stability in Car Following," Operations Research, Vol.7, pp.86-106, 1959.
- 34 Lee, G., "A Generalization of Linear Car-following Theory," Op. Res., Vol.14, pp.595-606, 1966.
- 35 Gazis, D.C., R. Herman, and R.B. Potts, "Car-following Theory of Steady State Traffic Flow," Operations Research, Vol.7, pp.499-505, 1959.
- 36 Edie, L.C., "Car-following and Steady-state Theory for Non-congested Traffic," Operations Research, Vol.9, pp.66-76, 1961.
- 37 Gazis, D.C., R. Herman, and R.W. Rothery, "Non-Linear Follow-the-Leader Models of Traffic Flow," Operations Research, Vol.9, pp.545-567, 1961.

- 38 May, Jr., Adolf D. and Hartmut E.M. Keller, "Non-Integer Car-following Models," Highway Research Record, No.199, pp.19-32, 1967.
- 39 Pipes, Louis A., "Car Following Models and the Fundamental Diagram of Road Traffic," Transportation Research, Vol.1, pp.21-29, 1967.
- 40 Herman, R. and R.B. Potts, "Single-lane Traffic Theory and Experiment," Theory of Traffic Flow, R.Herman, Ed., Elsevier, 1961.
- 41 Kometani, E. and T. Sasaki, "Dynamic Behavior of Traffic with a Non-linear Spacing-speed Relationship," Theory of Traffic Flow, R. Herman, Ed., Elsevier, 1961.
- 42 Newell, G., "Non-linear Effects in the Dynamics of Car-following," Op.Res., Vol.9, pp.209-229, 1961.
- 43 Herman, R. and R.W. Rothery, "Car-following and Steady State Flow," Proceedings of the Second International Symposium on the Theory of Traffic Flow, London, pp.1-11, June 1963.
- 44 Michaels, R.M., "Perceptual Factors in Car-following," Proceedings of the Second International Symposium on the Theory of Traffic Flow, London, pp.44-59, June 1963.
- 45 Athol, P., "Headway Groupings," HRR, No.72, pp.137-155, 1965.
- 46 Wagner, F.A. and A.D. May, "Volume and Speed Characteristics at Seven Study Locations," HRB Bull. 281, pp.48-67, 1960.
- 47 Drew, D., C. Dudek, and C. Keese, "Freeway Level of Service as Described by an Energy-Acceleration Noise Model," HRR 162, pp.30-85, 1967.
- 48 Surti, V. and E. Gervais, "Peak Period Comfort and Service Evaluation of an Urban Freeway and an Alternate Surface Street," HRR 157, pp. 144-178, 1967.
- 49 Greenshields, B., "Traffic Accidents and the Quality of Traffic Flow," HRB Bull. 208, pp.1-15, 1959.
- 50 Weinstock, R., "Calculus of Variations", p.61, McGraw Hill Book Company, 1952.

AUTOBIOGRAPHICAL STATEMENT

Mr. Stephen Kleinman was born in New York City in 1941.

After graduating from the Bronx High School of Science he attended Rensselaer Polytechnic Institute and received the Bachelor of Electrical Engineering degree in 1962. He pursued his interests in switching circuits and logical design at Columbia University where he attained the Master of Science degree in Electrical Engineering in 1964.

At this time Mr. Kleinman accepted an appointment as Lecturer on the staff of the Electrical Engineering Department of The City College of the City University of New York. After two years of teaching a wide range of courses he returned to graduate study at The City College. His current interest is in the application of system theory to urban problems.

Mr. Kleinman is a member of the IEEE, Eta Kappa Nu, The Highway Research Board, and the Operations Research Society of America.